

**From:** BIERY, PAUL KIM <PAUL.BIERY@scana.com>

**Sent:** Wednesday, October 9, 2019 8:00 AM

**To:** LAKhan@ColumbiaSC.Net <LAKhan@ColumbiaSC.Net>; cstone@lex-co.com <cstone@lex-co.com>; Heather Brown <Brown.Heather@richlandcountysc.gov>; coxM@dnr.sc.gov <coxM@dnr.sc.gov>

**Cc:** EFFINGER, THOMAS (SEG Services - 6) <THOMAS.EFFINGER@DOMINIONENERGY.COM>; Amy Cappellino (Amy.e.Cappellino@usace.army.mil) <Amy.e.Cappellino@usace.army.mil>; Cassidy, Greg <cassidga@dhec.sc.gov>; Berresford, James <berresjl@dhec.sc.gov>; Rusty Contrael <rcontrael21@outlook.com>

**Subject:** Dominion Energy South Carolina Congaree River Project (No-Rise Certification Submittal)

**\*\*\* Caution. This is an EXTERNAL email. DO NOT open attachments or click links from unknown senders or unexpected email. \*\*\***

Dear local Floodplain Managers:

Good morning. Dominion Energy South Carolina (DESC) is continuing to pursue a United States Army Corp of Engineers (USACE) permit to perform a Modified Removal Action (MRA) of a Tar-like Material (TLM) from the Congaree River in Columbia, SC [Attachment 1]. The MRA was reviewed and approved by SCDHEC and various Stakeholders on February 7, 2019 and a document describing the approved scope of work is attached [Attachment 2].

As you may recall, DESC commissioned WSP (our design Engineer) to perform a Hydraulic Analysis (i.e. backwater study) to determine the impact of the proposed cofferdam structures on the Base Flood Elevations (BFE) for existing conditions [Attachment 3]. Based on the backwater analysis, a signed "Engineering 'No-Rise' Certification" was prepared that demonstrated the proposed Congaree River Remediation Project will not impact the 100-year flood elevations, etc., for both Richland and Lexington counties [Attachments 4 & 5].

On Friday, October 4<sup>th</sup>, 2019 - DESC received notification from the USACE that their comments have been addressed and the analysis regarding the "Engineering No-Rise Certifications" is ready to be submitted to the Floodplain Managers. Therefore, DESC is officially submitting the following documents for your respective review and subsequent written approvals:

#### **Attachments**

- 1 – Final MRA 12-12-18 [i.e. Stakeholder-Developed Modified Removal Action (MRA)]
- 2 – Concurrence Letter for MRA with stakeholder declarations final 02-07-19
- 3 – Congaree River Hydraulic Analysis Report Memo dated April 12, 2019
- 4 – Congaree River Remediation No-Rise Certification – April 12, 2019 – Richland County
- 5 – Congaree River Remediation No-Rise Certification – August 13, 2019 – Lexington County
- 6 – A link to a directory with the Calculations used to develop the "No-Rise Certifications" <https://apexcos.sharepoint.com/:f/s/PittsburghPA/EkPfSEAzUyIQj12r-JC18G4Bs9ybkIx1LFSUCqhGV3ABhQ?e=9tuiu6>

These documents have not changed since the preliminary submittal provided earlier for your review. Based on the USACE's request, we are seeking Floodplain Coordinator approval(s) of the No-Rise Certifications so that we may proceed with additional engineering and permitting work to complete this important project. Please provide your written approval (on your respective letterheads) at your earliest convenience.

Please call or email with any questions or comments.

Thank you for your prompt attention to this request.

Paul Biery  
Senior Project Manager

400 Otarre Parkway, Cayce, SC 29033  
Mailing Address: 220 Operation Way, MC C221, Cayce, SC 29033  
O: 803-217-5016 M: 803-465-7736



December 12, 2018

Mr. Lucas Berresford, Program Manager  
State Voluntary Cleanup Program  
Division of Site Assessment, Remediation, and Revitalization  
Bureau of Land and Waste Management  
South Carolina DHEC  
2600 Bull Street  
Columbia, SC 29201

RE: DRAFT Conceptual Plan for a Modified Removal Action  
SCE&G Fleet Maintenance Site (Congaree River)  
Columbia, South Carolina  
VCC# 02-4295-RP

Dear Mr. Berresford:

At the stakeholder meeting held at DHEC's office on November 15<sup>th</sup>, 2018, consensus agreement was reached between stakeholders on a Modified Removal Action (MRA) as the preferred path forward on the Congaree River sediment remediation project. At the conclusion of that meeting, DHEC requested SCE&G to prepare drawings and provide a written summary of the MRA for DHEC approval and continuing stakeholder agreement. Attached hereto are the requested drawings and written summary, presented as the Congaree River Project Conceptual Plan for a Modified Removal Action-December 2018. (Conceptual Plan). SCE&G is prepared to submit a pre-application meeting request to the Army Corps of Engineers once DHEC has approved the Conceptual Plan and it has been accepted by Stakeholders.

SCE&G remains committed to pursue any DHEC preferred final remedy that can be conducted in a safe and effective manner. Thank you for your help to move a remedy forward for the Congaree River sediments. If you have questions, please don't hesitate to give me a call at 803-217-9367.

Sincerely,

A handwritten signature in blue ink that reads "Thomas N. Effinger".

Thomas N. Effinger, Director  
Corporate Environmental Services  
SCANA

cc: G. Cassidy,  
G. Kendall Taylor,  
Myra Reece,  
Veronica Barringer

## **Congaree River Project**

### **Conceptual Plan for a Modified Removal Action – December 2018**

#### **Objective**

The Conceptual Plan delineates a revised approach towards completing a Modified Removal Action (MRA) as the preferred final remedy to address impacted sediment, tar-like material (TLM) and any associated effects that may exist within the Congaree River in Columbia, SC. The Conceptual Plan also defines the agreement and understanding reached among the Stakeholders listed on this document. The Stakeholders attended several meetings at DHEC to discuss the Congaree River remediation project and to develop a potential path forward. The result of those meetings and discussions was consensus agreement on the proposed MRA on November 15, 2018:

- To pursue a Modified Removal Action (MRA) consisting of the removal of sediment and tar-like material (TLM) from two separate areas as depicted on the attached figures and estimated in the attached table as a revised approach in an effort to receive a favorable Department of the Army (DA) permit decision for the necessary cofferdam as well as all other required regulatory approvals.

This Conceptual Plan outlines the proposed limits of the MRA based on the attached figures and justification which forms the basis for all future permit applications, engineering studies, design submittals, etc. that will be required in pursuit of the permits and approvals necessary to implement the MRA.

#### **Conceptual Plan MRA – December 2018**

The MRA will involve removal of impacted sediments and TLM from targeted areas that are:

- Close to the shoreline and therefore more susceptible to human dermal contact or exposure (e.g. to recreational users such as kayakers, waders/swimmers, fishermen etc.); and
- More concentrated with TLM, or where thicker deposits of TLM are shown to exist.

The attached Figures provide visual representations of the areas targeted for a modified removal action and their location. Figure 1 compares the outline of the previously proposed full-scale removal action (PRAWP - September 2018) to the two areas comprising this proposed MRA. Also note that the volumes shown on Figure 1 for each approach were calculated using a combination of new survey information collected in the spring of 2018 and the sediment coring logs collected from the remedial investigations conducted in 2010 to 2012. Figure 2 shows the MRA areas with a GIS visualization of each sediment boring as a TLM “hot-spot” which depicts the greater thickness of the TLM by a brighter color. Figure 3 provides an updated depiction of the average TLM thickness with estimated volume, using a similar GIS tool in which the TLM representation extends into the adjacent data point. Sediments in the “other areas” that will not be removed consist of either:

- Relatively minor thicknesses of TLM, and/or
- Are now covered by additional sediment resulting from the “superstorm” of 2015; and/or

- Occur far enough away from the shoreline and in deeper water, whereby risk of human dermal contact or exposure is minimal.

The proposed MRA consists of two areas as shown on the attached Figures. Area 1 is approximately 2.6 acres and has a similar footprint to the original full-scale Phase 1 area. Area 2 is approximately 0.5 acres in size.

Table 1 provides a comparison of volume estimates from previously submitted documents. Assuming successful completion of the MRA as proposed in this Conceptual Plan, an estimated 73% of the total TLM is expected to be removed from the Congaree River.

### **Permitting**

Regulatory permits and approvals will be required from the US Army Corps of Engineers (USACE), various sections within SCDHEC, the SC Department of Natural Resources (SCDNR), the Federal Emergency Management Agency (FEMA) local floodplain coordinators, the City of Columbia and other related state and local authorities to conduct the proposed work within and adjacent to the Congaree River.

Although the Stakeholders acknowledge that significant permitting challenges exist and may prove to be insurmountable, it is the mutual desire among the Stakeholders that all appropriate permits and approvals be obtained so that the MRA can be implemented.

### **Stakeholders attending the DHEC meetings**

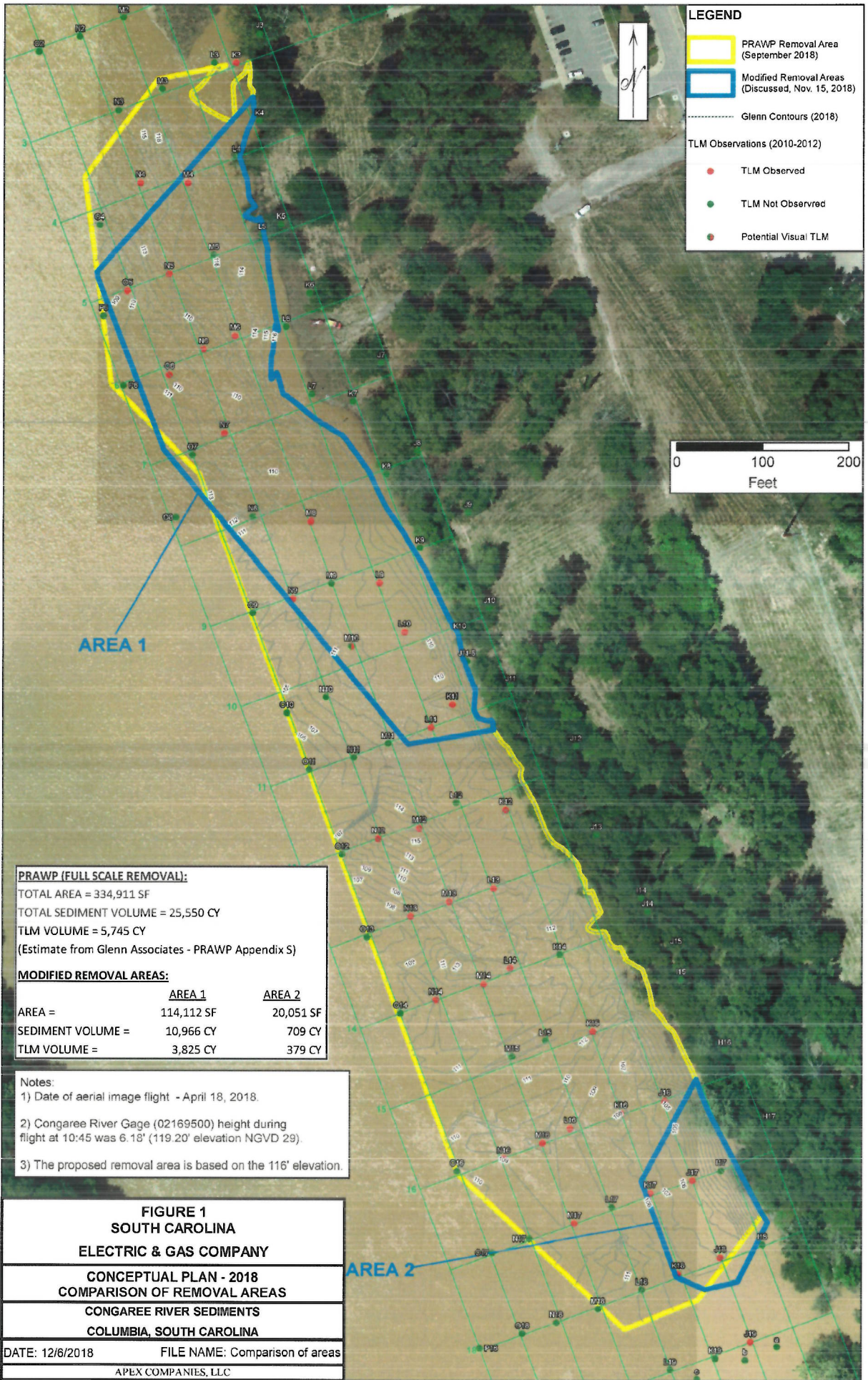
SCANA/SCE&G  
SCDHEC  
Congaree Riverkeeper  
Southern Environmental Law Center  
Guignard Associates  
City of Columbia  
Office of the SC Governor

### **Attachments**

Table 1  
3 Figures  
Meeting sign-in sheet

**Table 1 Estimated Removal Volumes**

Estimated by	MTR	Glenn & Associates	Apex	Percent Removal
	EE/CA Sediment Volume Estimate January 2013 Original, Full-Area Removal	Preliminary Removal Action Work Plan (PRAWP) September 2018 Full-Area Removal	Proposed Conceptual Plan - MRA Sediment Volume Estimate December 2018 MRA Areas 1 & 2	MRA vs PRAWP
<b>Total Volume of Sediment to be Removed</b>	<b>26,700 CY</b>	<b>25,550 CY</b>	<b>11,675 CY</b>	<b>46%</b>
<b>Total Volume of TLM to be Removed</b>	<b>Not Estimated</b>	<b>5,745 CY</b>	<b>4,204 CY</b>	<b>73%</b>



**PRAWP (FULL SCALE REMOVAL):**  
 TOTAL AREA = 334,911 SF  
 TOTAL SEDIMENT VOLUME = 25,550 CY  
 TLM VOLUME = 5,745 CY  
 (Estimate from Glenn Associates - PRAWP Appendix S)

**MODIFIED REMOVAL AREAS:**

	AREA 1	AREA 2
AREA =	114,112 SF	20,051 SF
SEDIMENT VOLUME =	10,966 CY	709 CY
TLM VOLUME =	3,825 CY	379 CY

- Notes:
- 1) Date of aerial image flight - April 18, 2018.
  - 2) Congaree River Gage (02169500) height during flight at 10:45 was 6.18' (119.20' elevation NGVD 29).
  - 3) The proposed removal area is based on the 116' elevation.

**FIGURE 1**  
**SOUTH CAROLINA**  
**ELECTRIC & GAS COMPANY**

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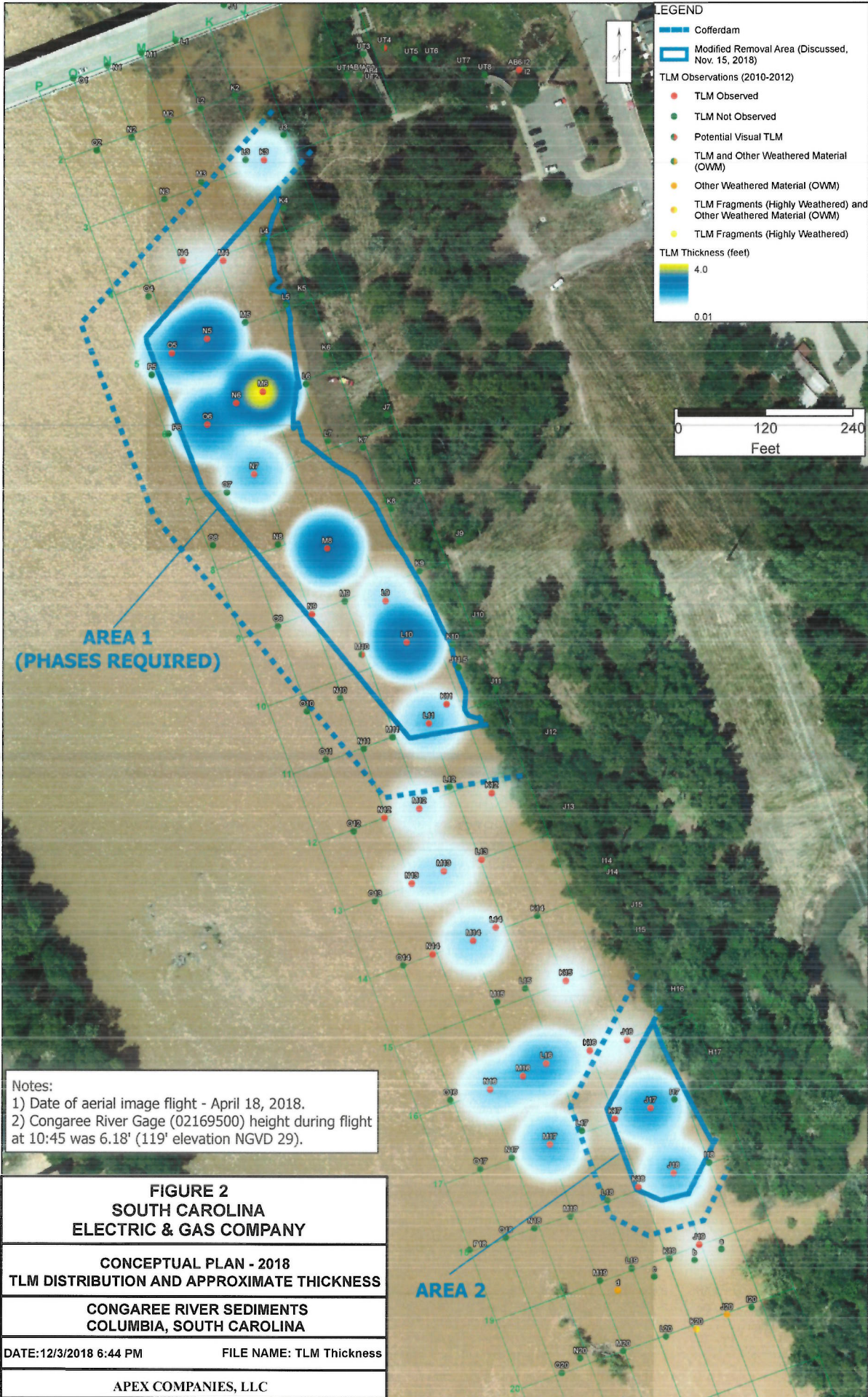
**CONCEPTUAL PLAN - 2018**  
**COMPARISON OF REMOVAL AREAS**  
**CONGAREE RIVER SEDIMENTS**  
**COLUMBIA, SOUTH CAROLINA**

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DATE: 12/6/2018      FILE NAME: Comparison of areas

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APEX COMPANIES, LLC



Notes:  
 1) Date of aerial image flight - April 18, 2018.  
 2) Congaree River Gage (02169500) height during flight at 10:45 was 6.18' (119' elevation NGVD 29).

**FIGURE 2**  
**SOUTH CAROLINA**  
**ELECTRIC & GAS COMPANY**

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**CONCEPTUAL PLAN - 2018**  
**TLM DISTRIBUTION AND APPROXIMATE THICKNESS**

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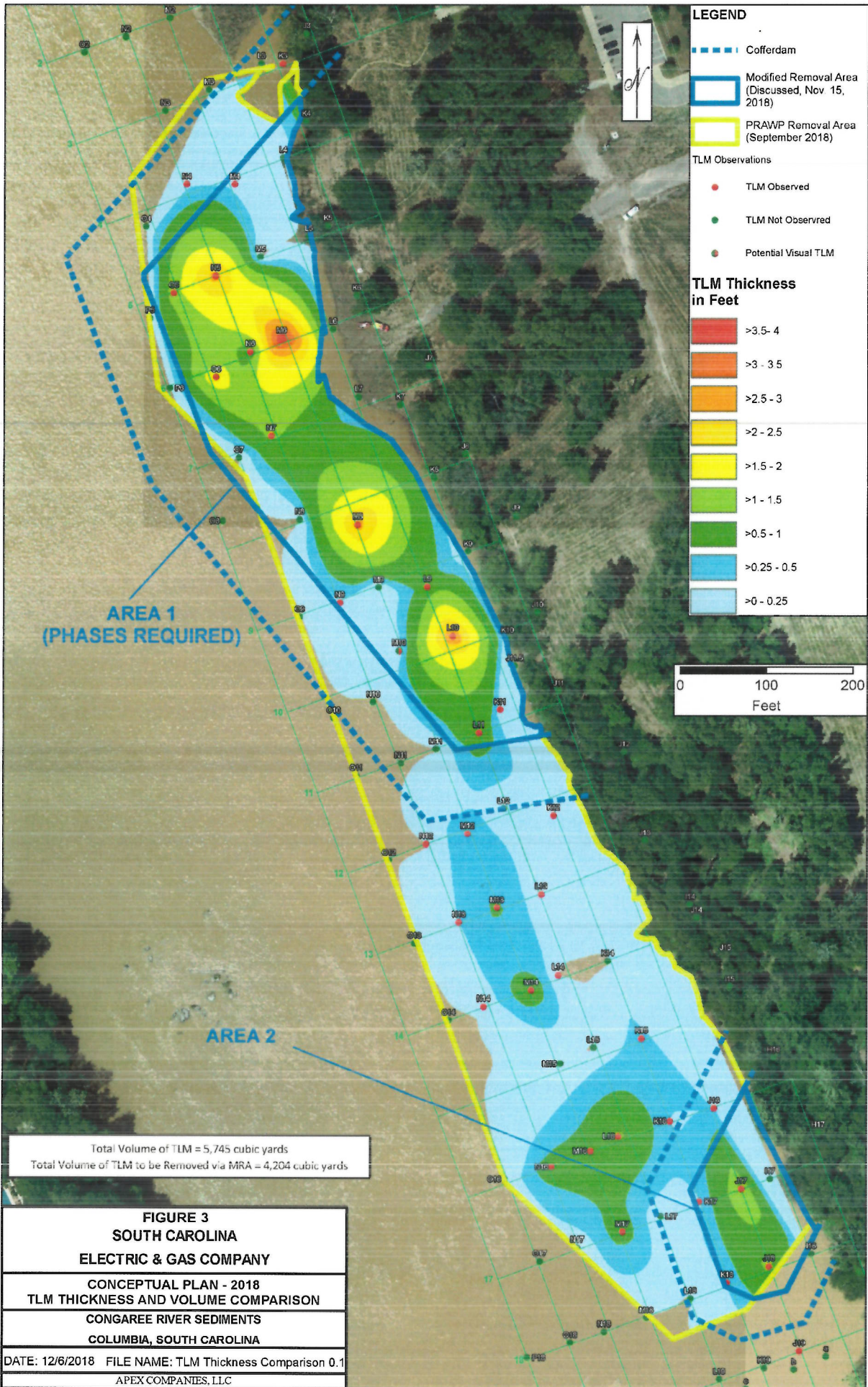
**CONGAREE RIVER SEDIMENTS**  
**COLUMBIA, SOUTH CAROLINA**

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DATE:12/3/2018 6:44 PM      FILE NAME: TLM Thickness

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APEX COMPANIES, LLC





# CONGAREE RIVER SEDIMENT

November 15, 2018

NO.	NAME	PHONE	EMAIL	ORGANIZATION
1	GREG CASSIDY	803-898-0910	cassidga@dhec.sc.gov	DHEC
2	Ken Taylor	803-898-0835	taylor.k@dhcc.sc.gov	DHEC
3	Henry Poche	803-898-1911	poche.h@dhcc.sc.gov	DHEC
4	Michael Trayham	803-898-0288	Michael.Trayham@dhec.sc.gov	DHEC
5	RUSTY CONTRAEL	412-721-0494	RCONTRAEL21@OUTLOOK.COM	SECEG CONSULTANT
6	TOMMY LAVENDER	803-253-8253	TLavender@the-southwest.com	Northwest - Congaree
7	CHARLES THOMPSON	803-254-2125	C.Thompson@THOMPSONSC.COM	GUILDARD ASSOCIATES LLC
8	Karen Retigan	803-898-1450	retigk@dhcc.sc.gov	DHEC
9	JONATHAN VANCEBOROUGH	803-217-6316	Jonathan.Vanceborough@sciana.com	SCANA
10	DAN SIRBY	803-898-1996	Sirbyd@dhec.sc.gov	SCDHEC
11	Steve Clark	803-600-2028	schk22@gmail.com	Congaree Release
12	Bill Stangor	803-760-3357	bill@congariverrestoration.org	CRF
13	Liz Jones	813-619-4169	ejones@selc.org	Southern Env. Law Ctr
14	Catherine Wannamaker	"	cwannamaker@selc.org	SELC
15	Mark Plondon	803-734-0522	mplondon@govmwr.sc.gov	NOV Office

# CONGAREE RIVER SEDIMENT

November 15, 2018

NO.	NAME	PHONE	EMAIL	ORGANIZATION
1	Hugood Hamilton	803-217-8938	ihamilton@scana.com	SEEG
2	Myla Bree	803/898-4102	resc@scana.com	SCDHEC
3	Tom Evinger	803/217-9367	teffinger@scana.com	SEEG
4	Rusty Contrael	412/721-6994		
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February 7, 2019

Mr. Thomas Effinger  
Southeast Energy Group (SEG)  
Mail Code C-221  
220 Operation Way  
Cayce, SC 29033-3701

RE: Draft Conceptual Plan for a Modified Removal Action (dated 12/12/18)  
SCE&G Fleet Maintenance Site (Congaree River)  
Columbia, South Carolina

Dear Mr. Effinger:

The Department (DHEC) has reviewed the Modified Removal Action (MRA) approach presented in the Draft Conceptual Plan and agrees this plan should be considered the preferred path-forward on the Congaree River sediment remediation project being conducted under DHEC's Responsible Party Voluntary Cleanup Contract (VCC 02-4295-RP.) This approach would target for removal areas where tar like material (TLM) is most prevalent and poses the greatest risk to human exposure. The target areas are shown in the three figures attached. It also acknowledges that some TLM might remain in the riverbed but situated in areas that would pose very little future risk to human exposure.

DHEC has consulted with two primary stakeholders in this project - The Congaree Riverkeeper and Guignard Associates LLC - and they too have expressed their support of this modified Removal Plan (see attached supporting statements for the record from each attached).

The Department requests SCE&G carry this proposal forward and draft all the proper documents to achieve the necessary regulatory permits and approvals required for implementation. Additionally, understanding that this project is being conducted under DHEC's Responsible Party Voluntary Cleanup Contract (VCC 02-4295-RP), stakeholder involvement and public participation will occur through the entire project. DHEC and the stakeholders agree that this project should be accomplished by the most expeditious route possible in working with the U.S. Army Corps of Engineers.

Sincerely,

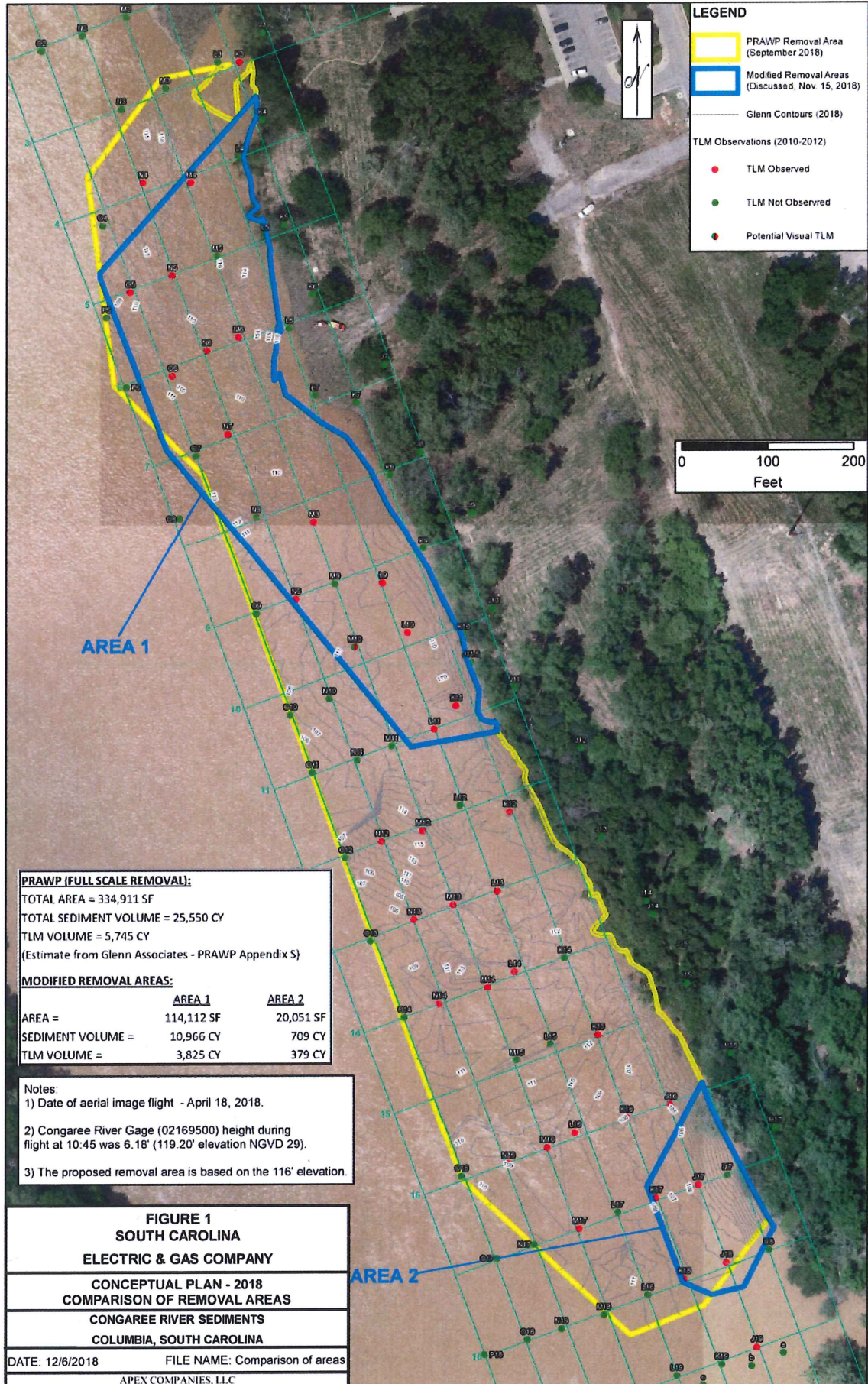
G. Kendall Taylor, Director  
Division of Site Assessment, Remediation, and Revitalization  
Bureau of Land and Waste Management

cc: File 52561  
Myra C. Reese, EA Director  
Veronica Barringer, Midlands EA Region  
Bill Stangler, Congaree Riverkeeper  
Charlie Thompson, Guignard Associates LLC

## Figures

**LEGEND**

- PRAWP Removal Area (September 2018)
- Modified Removal Areas (Discussed, Nov. 15, 2018)
- Glenn Contours (2018)
- TLM Observed
- TLM Not Observed
- Potential Visual TLM



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TOTAL AREA = 334,911 SF  
TOTAL SEDIMENT VOLUME = 25,550 CY  
TLM VOLUME = 5,745 CY  
(Estimate from Glenn Associates - PRAWP Appendix S)

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- Notes:**
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  - 3) The proposed removal area is based on the 116' elevation.

**FIGURE 1**  
**SOUTH CAROLINA**  
**ELECTRIC & GAS COMPANY**

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**CONCEPTUAL PLAN - 2018**  
**COMPARISON OF REMOVAL AREAS**

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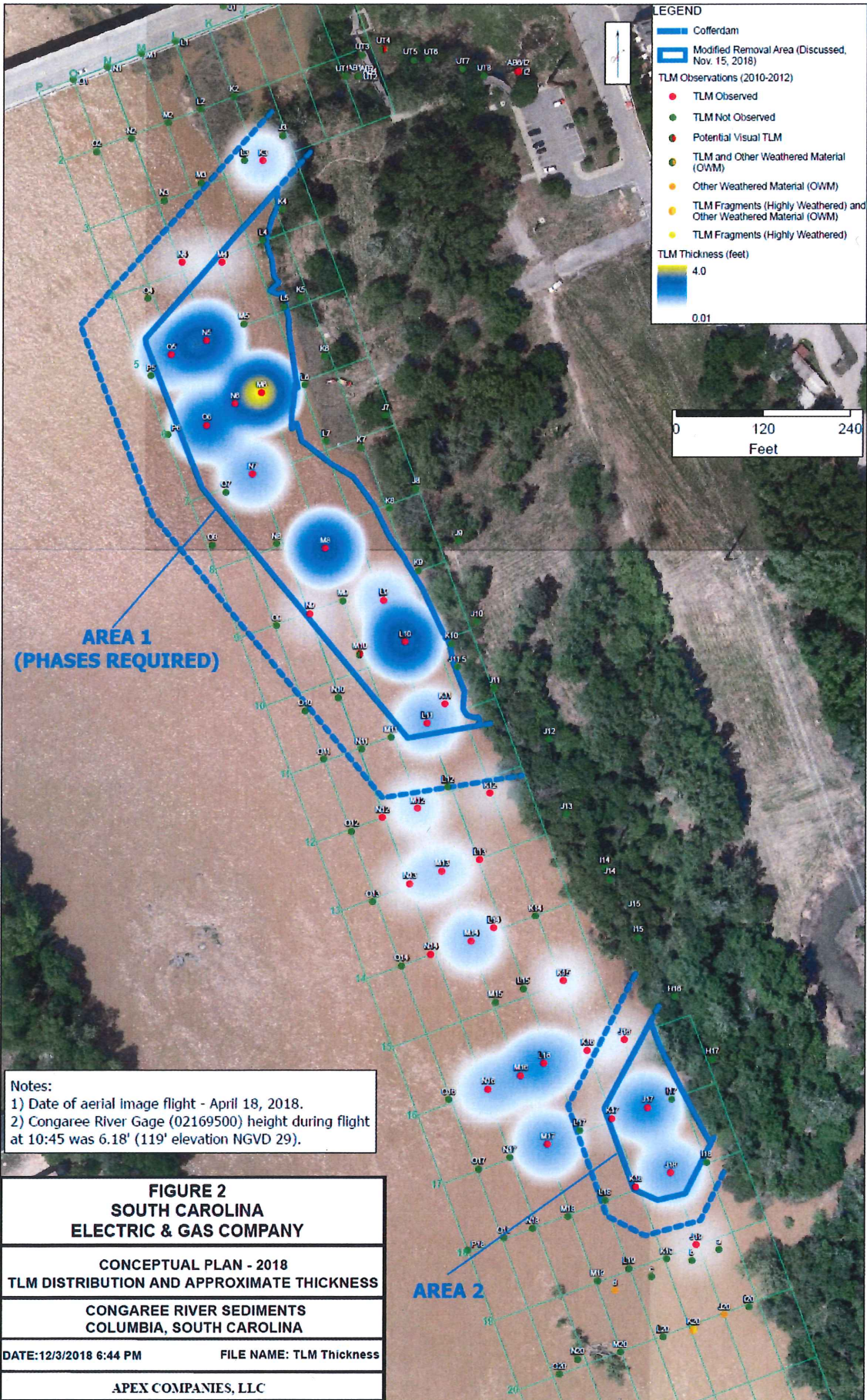
**CONGAREE RIVER SEDIMENTS**  
**COLUMBIA, SOUTH CAROLINA**

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DATE: 12/6/2018      FILE NAME: Comparison of areas

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APEX COMPANIES, LLC



**LEGEND**

- ▬ Cofferdam
- Modified Removal Area (Discussed, Nov. 15, 2018)
- TLM Observations (2010-2012)**
  - TLM Observed
  - TLM Not Observed
  - Potential Visual TLM
  - TLM and Other Weathered Material (OWM)
  - Other Weathered Material (OWM)
  - TLM Fragments (Highly Weathered) and Other Weathered Material (OWM)
  - TLM Fragments (Highly Weathered)
- TLM Thickness (feet)**
  - 4.0
  - 0.01

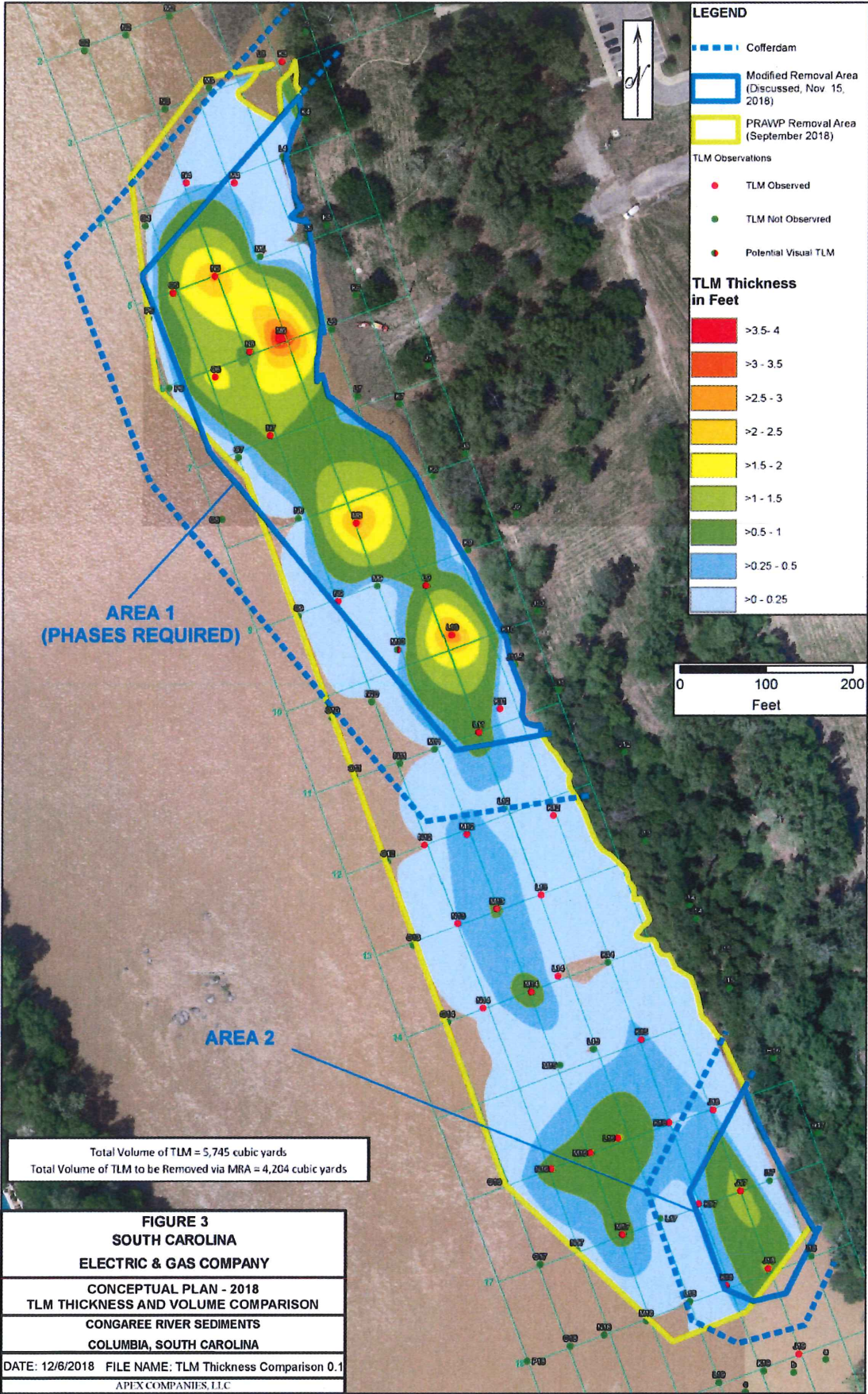


**AREA 1  
(PHASES REQUIRED)**

**AREA 2**

**Notes:**  
 1) Date of aerial image flight - April 18, 2018.  
 2) Congaree River Gage (02169500) height during flight at 10:45 was 6.18' (119' elevation NGVD 29).

<b>FIGURE 2 SOUTH CAROLINA ELECTRIC &amp; GAS COMPANY</b>	
CONCEPTUAL PLAN - 2018 TLM DISTRIBUTION AND APPROXIMATE THICKNESS	
CONGAREE RIVER SEDIMENTS COLUMBIA, SOUTH CAROLINA	
DATE: 12/3/2018 6:44 PM	FILE NAME: TLM Thickness
APEX COMPANIES, LLC	





Declaration of Support by Stakeholders:

- 1) Congaree Riverkeeper endorses the modified removal action presented by SCE&G and described above, with the understanding that SCE&G will work diligently and make all reasonable efforts to acquire the necessary permits and complete this proposed removal action, and SCE&G will provide stakeholders with regular updates on their progress. We are also committed to supporting the permitting strategy the Corps determines is most appropriate for the Modified Removal Action, provided that the permit authorization process is as expeditious as possible while still affording sufficient opportunity -- either within the permitting process or through the parallel voluntary cleanup contract led by DHEC -- for public comment and for agency consultation.

*William J. Stangler* (e-signature, 1/23/19)

Bill Stangler  
Congaree Riverkeeper

Declaration of Support by Stakeholders:

1. That any such plan and associated application be submitted to the USACE for authorization under the appropriate Nationwide permit, e.g., NWP-38, and not under an Individual permit, to expedite the process and help mitigate the total project timeline.



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Charles C. Thompson, Manager  
Guignard Associates LLC



## Statement of Purpose

The purpose of this calculation is to perform a hydraulic analysis for the affected area along the Congaree River in Columbia, South Carolina, due to the separate installation of two rock fill cofferdams around Areas 1 and 2. The purpose of performing a hydraulic analysis is to determine the impact of the proposed cofferdam structures on the Base Flood Elevations (BFE) for existing conditions.

A plan view showing the extents of the cofferdams is included on Figure 1, based on Apex Drawing "Stakeholder Approved MRA Plan Sediment Remediation Areas" (Apex, 2019a).

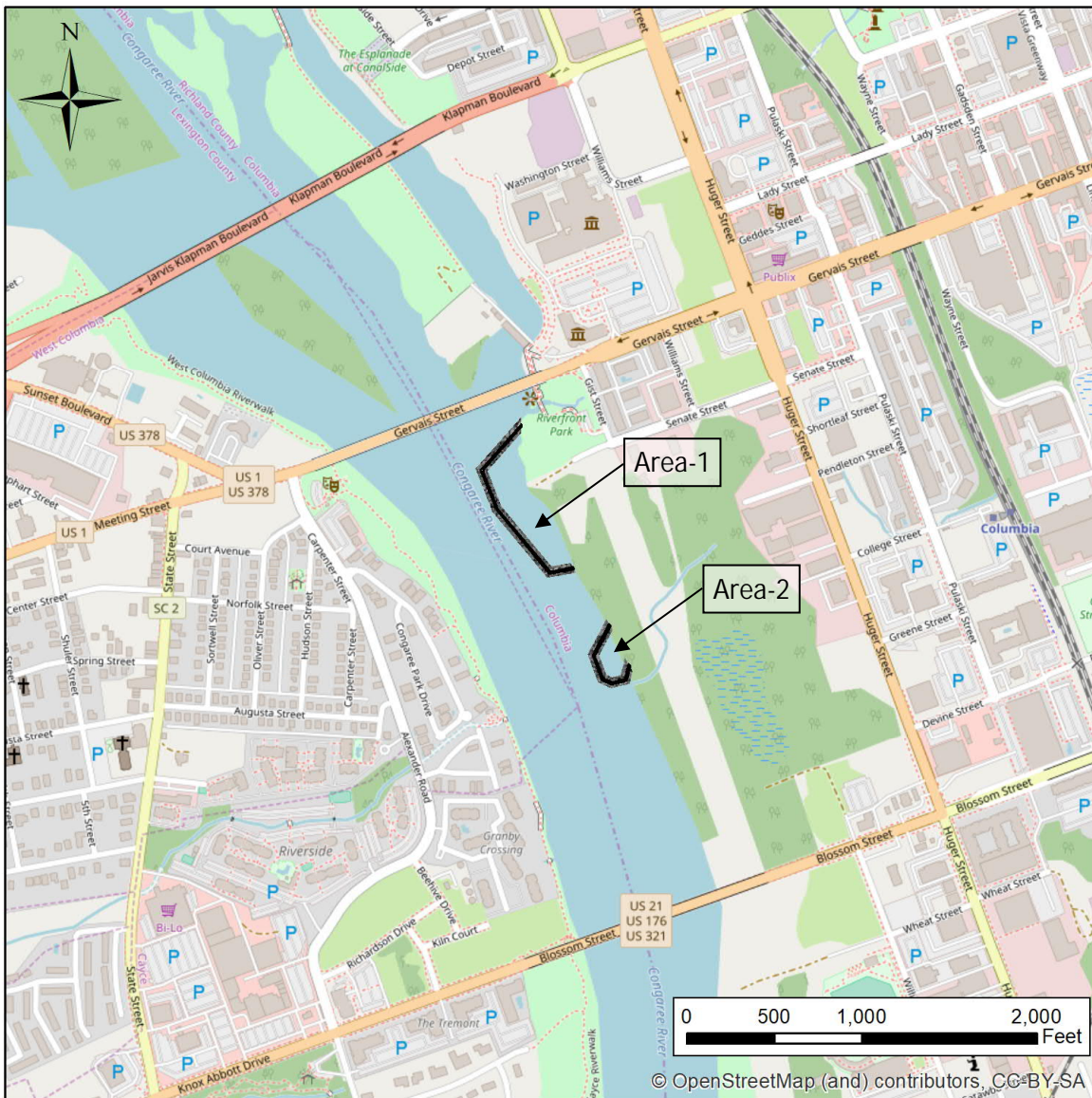
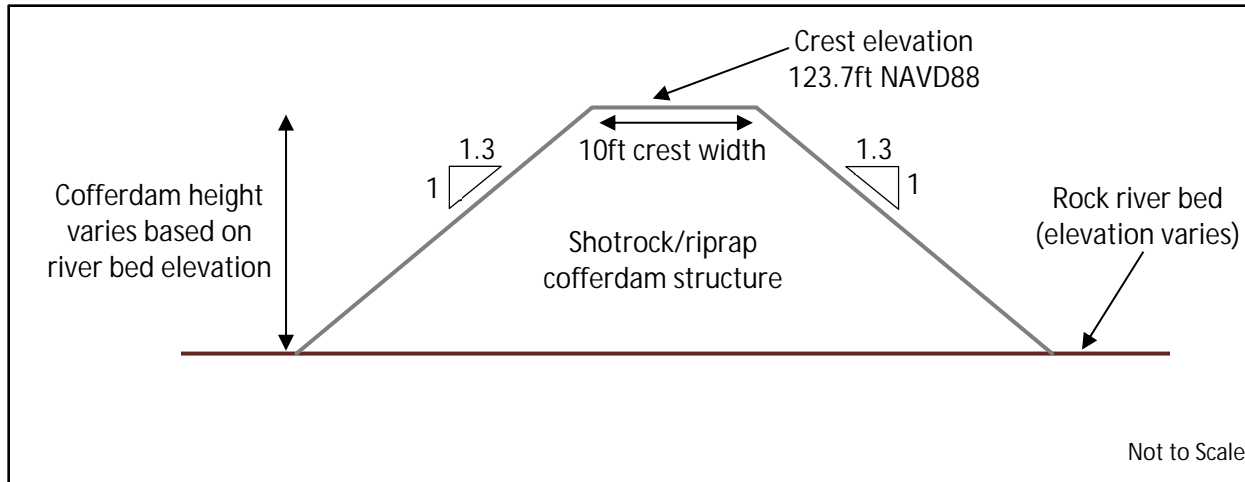


Figure 1: Plan View of Proposed Cofferdams

The typical section of the proposed cofferdam structures is shown in Figure 2. This is an assumed design concept based on discussions with Apex Companies LLC.



*Figure 2: Typical Section of Proposed Cofferdams*

## Description of Methodology Used

The installation of a rock fill berm cofferdam is proposed along the east bank of the Congaree River, starting just downstream of the Gervais Street Bridge. The proposed cofferdam will be constructed in two separate phases; therefore, each phase is modeled separately for the proposed conditions. The total cofferdam influence area runs approximately 1,650 feet along the east bank, starting downstream of the Gervais Street Bridge and terminating at the inlet of a small unnamed tributary, referred to as Tributary No. 2 for this analysis.

The hydraulic study follows procedures set forth by the Federal Emergency Management Agency (FEMA) in their "Procedures for No-Rise Certification" (FEMA, 2013), which is included in Appendix A. The No-Rise procedures follow several distinct steps:

1. Current Effective Model: Obtain a copy of the current effective hydraulic model for the specified stream from FEMA;
2. Duplicate Effective Model: Upon receipt of the effective model, run the model to duplicate the data in the effective FEMA Flood Insurance Study (FIS; FEMA, 2017/2018).
3. Corrected Effective Model: The model that corrects any errors that occur in the duplicate effective model, adds any additional cross sections, or incorporates more detailed topographic information than that used in the current effective model;
4. Existing Conditions Model: Revise the duplicate effective or the corrected effective model to reflect any modifications that have occurred within the floodplain since the date of the effective model but prior to the construction of the project. If no modifications have occurred since the date of the effective model, then the model would be identical to the duplicate effective or corrected effective model. The results of this Existing Conditions analysis will indicate the 100-yr elevations at the project site;
5. Proposed, or Post-Project Conditions Model: Modify the existing condition or pre-project conditions model (or duplicate effective model or corrected effective model, as appropriate) to

reflect proposed or post-project conditions. (this analysis looks at two separate proposed conditions models) The results of this analysis will indicate the 100-year elevation for proposed conditions at the project site.

### Current Effective Model

The current effective model was requested from FEMA by following the procedure outlined in the "Procedures for No-Rise Certification" (FEMA, 2013). The latest hydraulic model used in developing the current FIS for the Congaree River was requested. The hydraulic analyses in the FIS were carried out to estimate flood elevations of the selected recurrence interval. In this case, the recurrence interval is the 100-year flood. This means that the flood has a 1 percent chance of being equaled or exceeded during any given year.

The Congaree River flows along the boundary between Lexington and Richland Counties. A Flood Insurance Rate Map (FIRM) is available for both Lexington County (FEMA, 2018), and Richland County (FEMA, 2017) which includes the location of the cross-sections used to develop the hydraulic model. The two FIRMs are presented on Figure 3 and Figure 4 respectively.

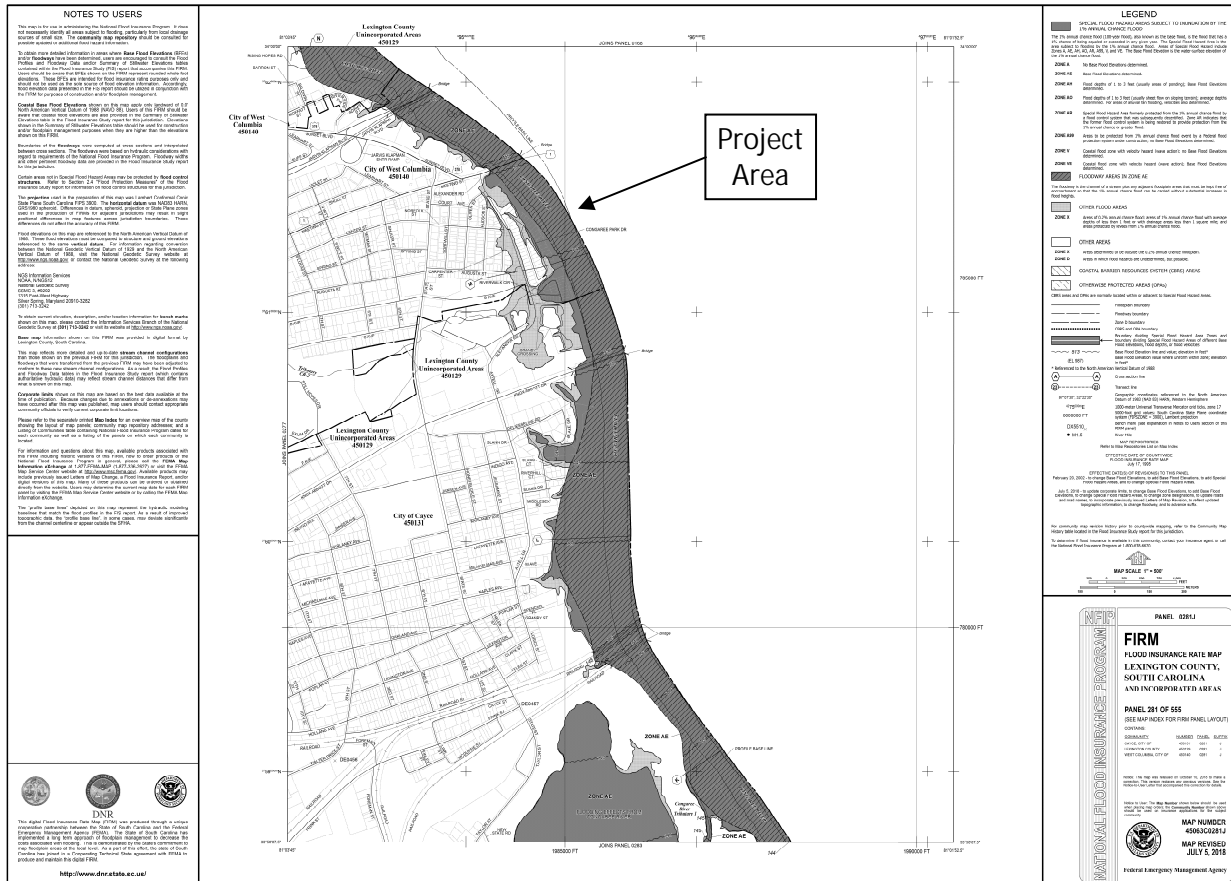


Figure 3: Lexington County Flood Insurance Rate Map (FEMA, 2018)

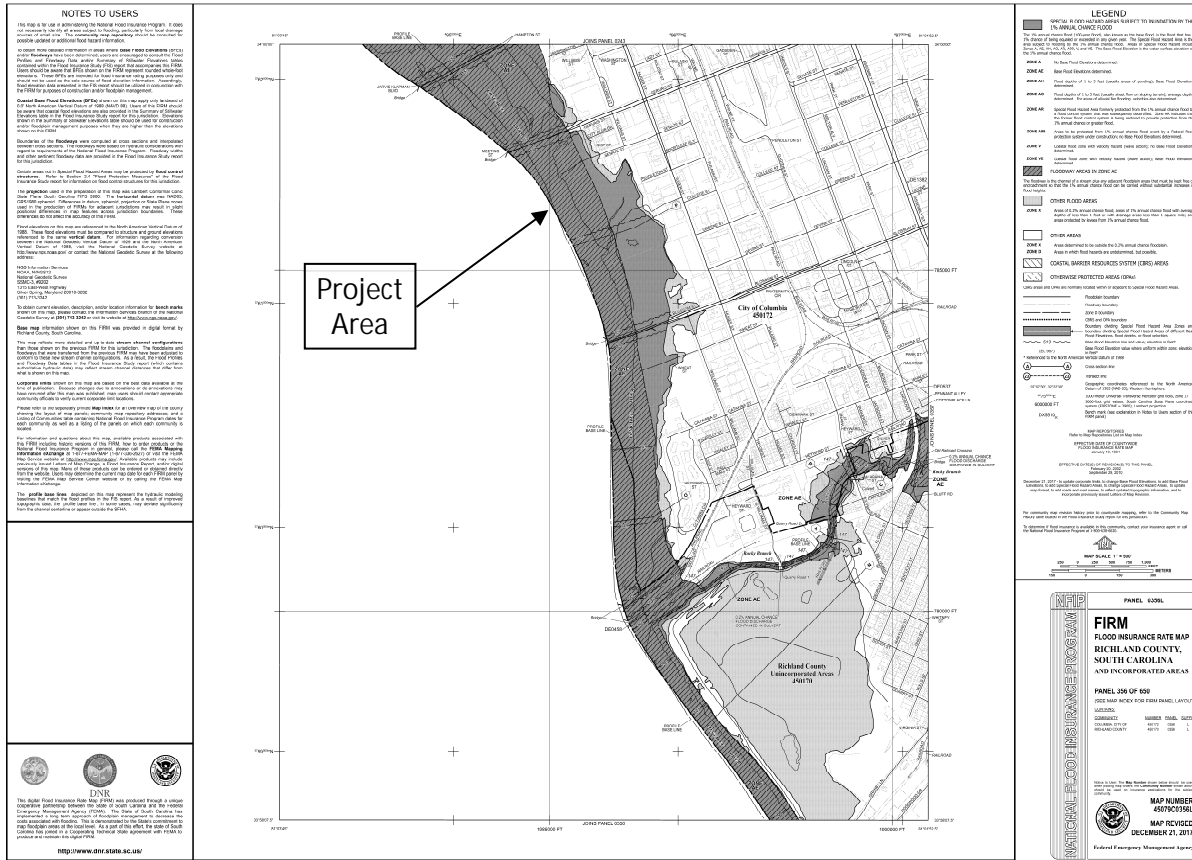


Figure 4: Richland County Flood Insurance Rate Map (FEMA, 2017)

The current effective model for the Congaree River was provided to WSP USA by FEMA in the form of electronic HEC-RAS (USACE, 2010) input and output files. The model is an unsteady flow model covering approximately 14 miles of the Congaree River. The vertical datum of the model is to the North American Vertical Datum of 1988 (NAVD88). The electronic files for all models are included in Appendix B.

### Duplicate Effective Model

HEC-RAS Version 4.1 (USACE, 2010) was used to develop the current effective model. HEC-RAS 4.1 is a hydraulic modeling program used for simulating one-dimensional steady and unsteady flows in river channels. The same software version was also used for the duplicate effective model, and all other models used in this hydraulic analysis, to maintain consistency with the current effective model.

Unsteady model simulations for the 10-year, 50-year, and 100-year flood return periods were completed for the duplicate effective model. No changes were made to the current effective model provided by FEMA; the model was not truncated or modified in any way. The results obtained were identical to the current effective model. The current/duplicate effective HEC-RAS model schematic is shown on Figure 5 (full model extent) and Figure 6 (project area).

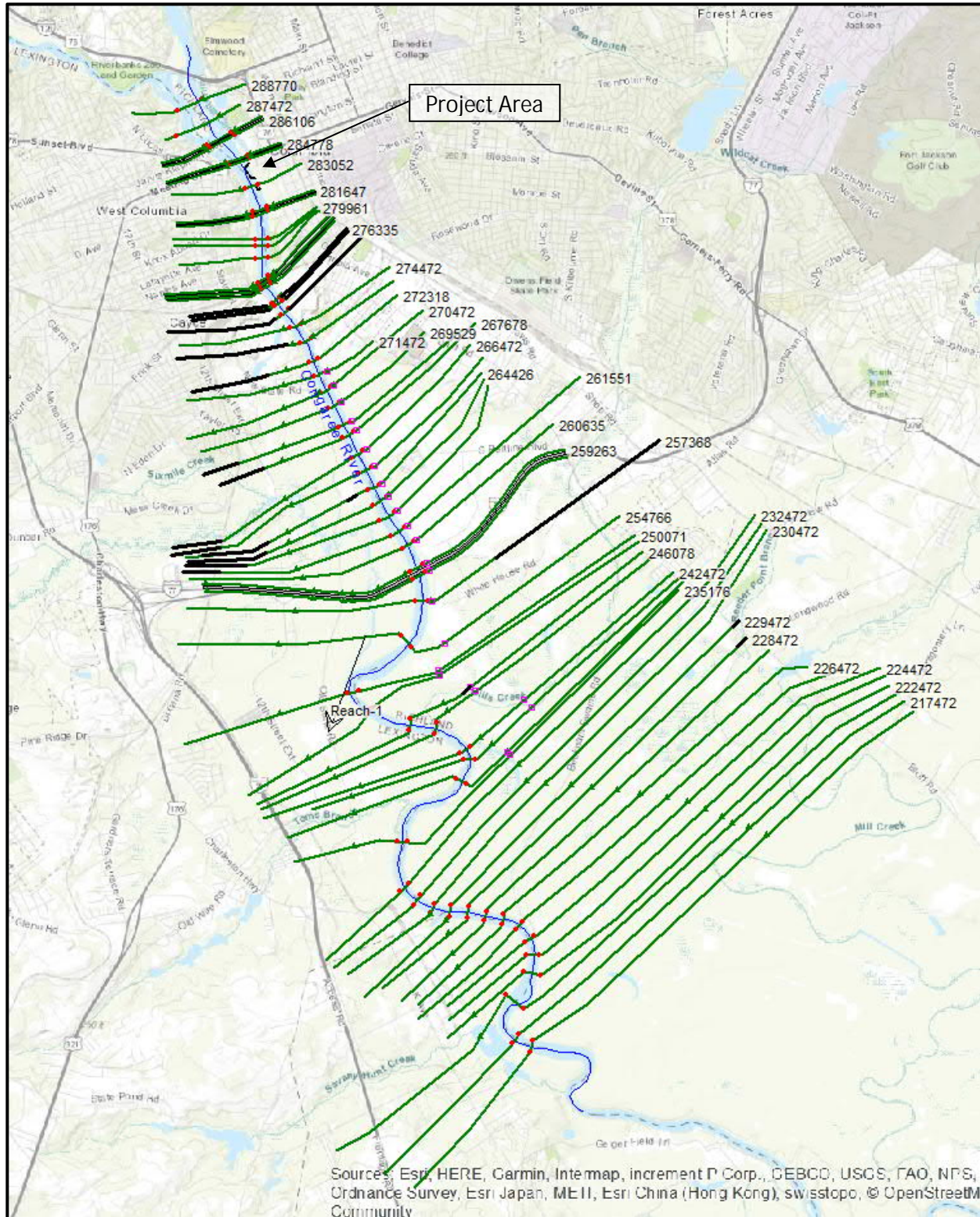
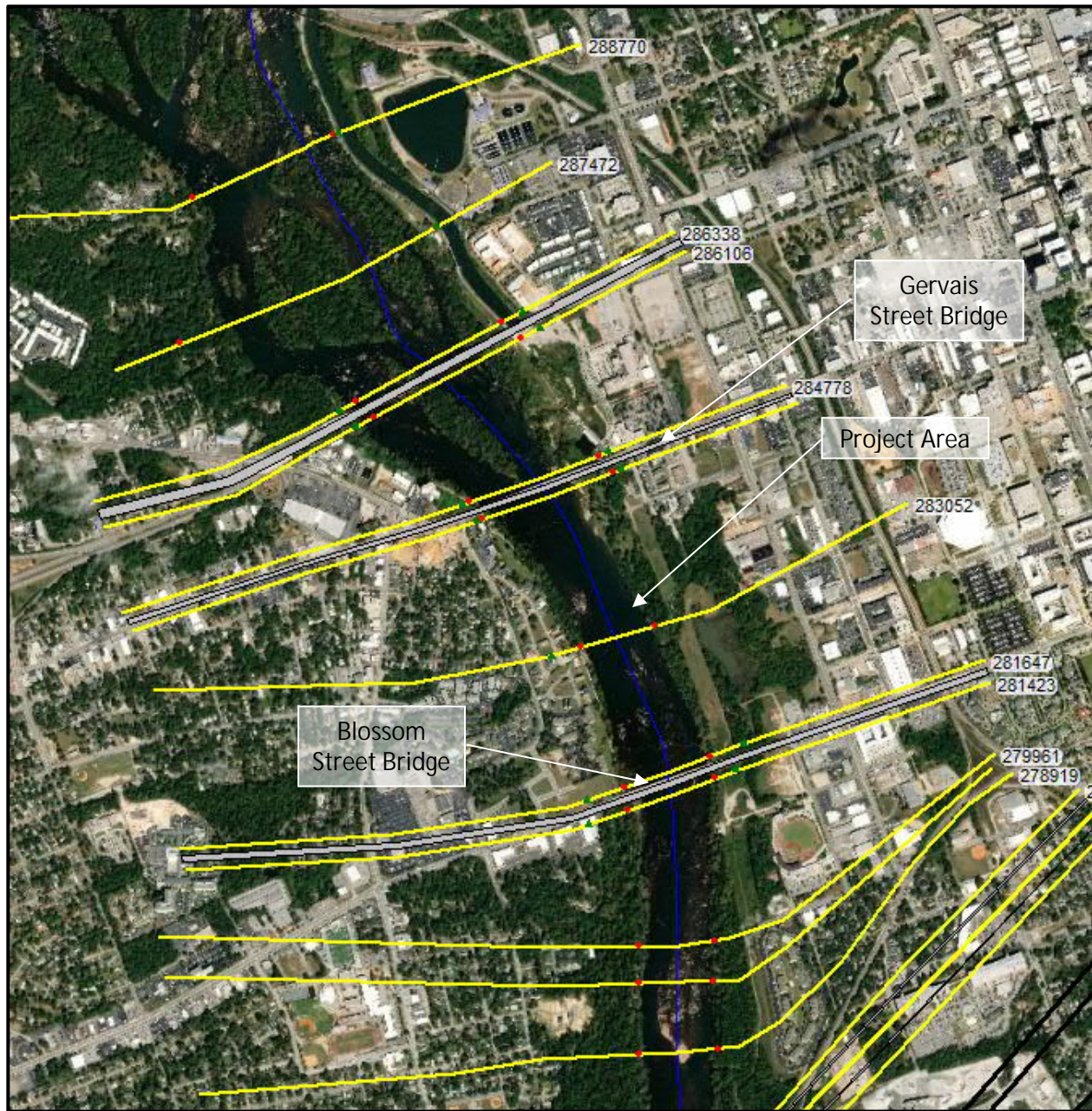


Figure 5: Current/Duplicate Effective HEC-RAS Model Schematic (Full Model Extent)



*Figure 6: Current/Duplicate Effective HEC-RAS Model Schematic (Project Area)*

### Corrected Effective Model

No errors were detected in the current/duplicate effective models, but additional topographic data was available for the project area which was used to update the corrected model between Gervais Street and Blossom Street bridges. Additional cross sections were also added in order to provide a more accurate representation of the channel conveyance and floodplain storage throughout the project area. The cross sections were specified in appropriate locations to allow representation of the cofferdams structures in the proposed conditions model.

Figure 7 shows the locations of the cross sections in the corrected effective model throughout the project area. A total of twenty-six additional cross sections were included between Gervais Street and Blossom Street bridges.



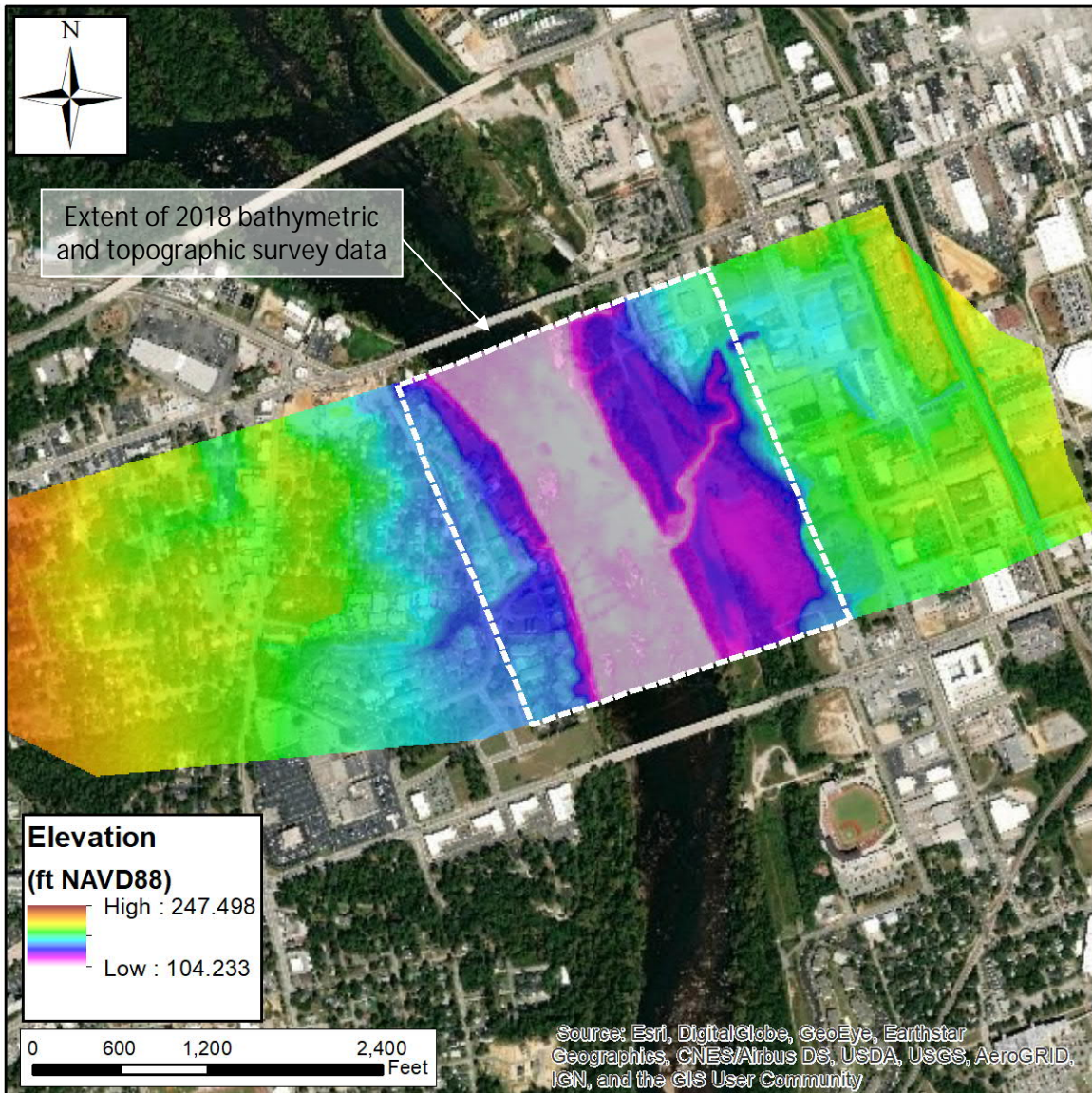
This number of sections was required to ensure accurate representation of the upstream and downstream extents of the cofferdams at Area-1 and Area-2.



*Figure 7: Corrected Effective HEC-RAS Model Schematic (Project Area)*

A bathymetric and topographic survey was completed for the project area in April through July 2018, which was provided to WSP as Drawing ACAD-13951-COMBINED.dwg (Apex, 2019b). The original data is referenced to the National Geodetic Vertical Datum of 1929 (NGVD29) and was converted to NAVD88 by applying the -0.787ft conversion determined from the National Oceanic and Atmospheric Administration (NOAA) Vertcon tool (NOAA, 2019). This process was necessary to ensure that the corrected effective model was updated with data referenced to a consistent datum.

LIDAR data downloaded from South Carolina Department of Natural Resources (SCDNR, 2010) was used to supplement the updated 2018 topographic and bathymetric data provided for the project. This was required to complete the Digital Elevation Model (DEM) for extended floodplain areas that the 2018 data did not cover. Figure 8 shows the updated DEM used to update the corrected effective hydraulic model. The area marked with a white dashed box is the extent of the 2018 project specific data, other parts of the DEM are based on the SCDNR LIDAR data, which was provided referenced to NAVD88.



*Figure 8: Updated Topographic and Bathymetric Data Extents*

The twenty-six new cross sections along with the one existing cross section (River Station 283342) shown on Figure 7, were cut from the DEM to create the corrected effective model. The cross-sections from the corrected model are shown in Appendix C, which also includes the existing cross sections immediately upstream (River Station 284565) and downstream (River Station 281647) of the project area, which were not modified for this analysis.

### Existing Conditions Model

No additional changes from the corrected model were required to represent the existing conditions. Therefore, an existing conditions model was not used for this analysis and the results from the corrected model are used to define the existing BFEs at the project site, and for subsequent comparison with the proposed conditions model.



## Proposed Conditions Model

The proposed conditions model includes modified cross-sections where the proposed cofferdams will be located. In this case, the proposed cofferdam is along the east bank of the Congaree River. As shown in the typical section (Figure 2), the proposed rockfill cofferdam has a 10-ft wide crest at elevation 123.7 ft (NAVD88) and a side slope of 1.3H:1.0V on both the inboard and outboard slopes.

The cofferdam is to be installed in two separate phases. Therefore, for the purposes of this analysis two separate proposed conditions models are analyzed; one for each of the two phases (Area-1 and Area-2, shown on Figure 1).

Each model is run and the results are compared to those of the corrected effective model to determine if the proposed conditions satisfy the No-Rise condition. Cross-sections updated for the proposed condition models are included in Appendix D.

## Assumptions and Justification

1. All elevations are referenced to NAVD88. Topographic survey data and USGS gage data was originally provided referenced to NGVD29, and was converted by applying the -0.787ft conversion determined from the NOAA Vertcon tool (NOAA, 2019).
2. The proposed Area-1 and Area-2 cofferdams are analyzed as a separate proposed conditions model, to reflect the phased approach being followed for the project.
3. The cross section of the proposed cofferdam structures is shown in Figure 2. This is an assumed design concept based on discussions with Apex Companies LLC.

## Calculation Input

The current effective model for the Congaree River was provided to WSP USA by FEMA in the form of electronic HEC-RAS (USACE, 2010) input and output files. The model is an unsteady flow model covering approximately 14 miles of the Congaree River. HEC-RAS Version 4.1 (USACE, 2010) was used to develop the current effective model. The same software version was also used for the duplicate effective model, and all other models used in this hydraulic analysis, to maintain consistency with the current effective model.

As discussed in previous sections, changes have been made to the duplicate effective model to create the corrected model. Further specific details are included in the following sections. No changes have been made to the hydraulic model outside of the project area, i.e., no changes have been made to cross sections between River Stations 288770 to 284565, and River Stations 281647 to 216472 (inclusive).

## Channel Cross Section and Structure Geometry

The geometric data for the twenty-six additional cross sections that were included between Gervais Street and Blossom Street bridges (shown on Figure 7) was extracted from the DEM developed for the project (shown on Figure 8). The geometry for one existing cross section, River Station 283042, was also extracted from the DEM and updated. No changes were made to the bridge structures, as the cross sections immediately upstream and downstream of these structures were not modified.

The cross section geometry was manually modified to represent the cofferdam structures within Area-1 and Area-2, based on Apex Drawing "Stakeholder Approved MRA Plan Sediment Remediation Areas" (Apex, 2019a). The upstream and downstream extents of the cofferdams are represented using four cross sections each. For the



upstream extent, the first cross section defines the geometry immediately upstream of the start of the cofferdam, and represents conditions at the upstream toe of the structure. No changes are made to this section for the proposed models. The next two cross section represent the crest of the structure that is perpendicular to the flow in the river channel. These sections are located 10ft apart to represent the 10ft crest width of the cofferdam. The crest of the upstream end of the cofferdam is intended to act as an overtopping structure, and it has a crest level set 1ft lower than the rest of the structure, at Elevation 122.7ft NAVD. The fourth cross section represents the topography at the downstream toe of the upstream cofferdam extent. This section also defines the geometry of the cofferdam that runs parallel to the river flow direction.

A series of cross sections is then used to represent the parts of the cofferdam(s) that are aligned parallel to the flow in the river. These sections define the crest of the cofferdam at Elevation 123.7ft NAVD88. This elevation is also defined as a levee crest feature; meaning that the area behind the cofferdam remains dry until the water rises above the crest and flows over the top of the structure. The storage and conveyance associated with the area behind the cofferdams is therefore not accounted for until the levee is overtopped. The dry area behind the cofferdams is also specified as an ineffective flow area to ensure that the additional cross-sectional area and wetted perimeter are not accounted for until the water level rises above the crest of the cofferdam.

The downstream extent of the cofferdam is also represented using the four cross section approach outlined above, with a cross section to represent the topography immediately upstream and downstream of the cofferdam, and two sections located 10ft apart to represent the crest width of the structure.

### Ineffective Flow Areas

The FEMA model uses ineffective flow areas to represent areas of the floodplain which only provide flood storage and not flow conveyance. The same approach has been applied for the new cross sections, with areas of the right and left overbanks specified as ineffective flow areas until the water level rises above specified elevations.

### Manning's Roughness Coefficient

The FEMA model uses a Manning's roughness coefficient of 0.038 for the main channel. For the left and right overbanks, a Manning's roughness coefficient of 0.1 or 0.125 is used to represent buildings in urban areas, and values of 0.08 to 0.04 to represent vegetated floodplain areas. These roughness coefficients were applied throughout the new cross sections; with a 0.038 value used to represent the main channel, 0.08 used to represent densely vegetated floodplain areas, and 0.125 used for urban areas with buildings. No changes were made to the roughness values between River Stations 288770 to 284565, and River Stations 281647 to 216472 (inclusive).

### Contraction and Expansion Coefficients

The FEMA model uses a contraction coefficient of 0.3 and expansion coefficient of 0.5 at the cross-sections just prior to or after any bridges in the model. Coefficients of 0.1 and 0.3, respectively, are used elsewhere throughout the model, and were applied to all new cross sections in the model.

### Boundary Conditions

The boundary conditions from the current effective model have been used for all model runs completed for this hydraulic analysis without modification. Time varying flow vs time inflow boundaries are used as the upstream model boundary, and a normal depth boundary based on a channel slope of 0.0004 is specified as the downstream boundary. The downstream boundary is approximately 13 miles downstream of the project area and will not be controlling the model results throughout the area of interest.

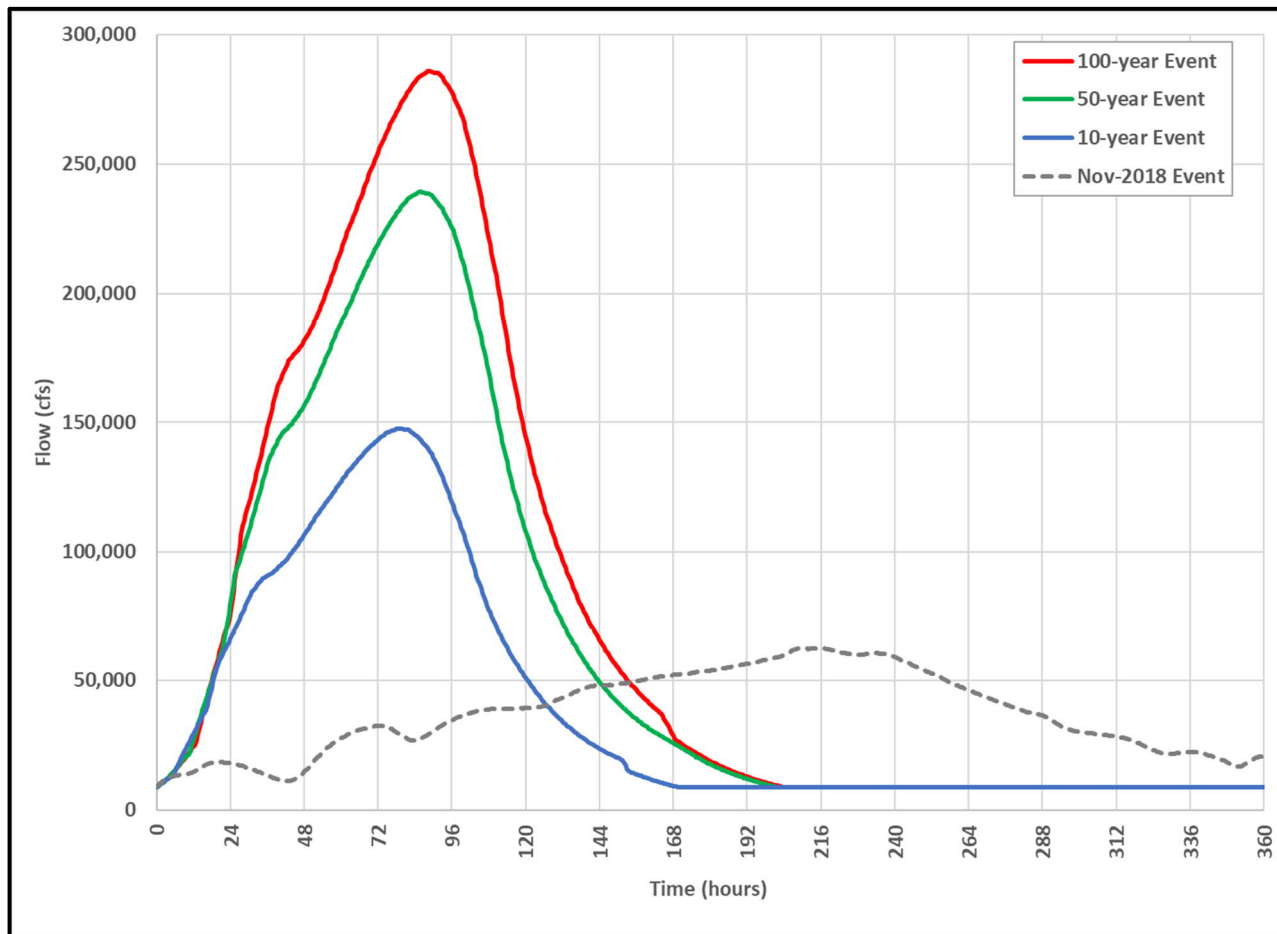


The peak flow values for the 100-year, 50-year, and 10-year flood events are provided in Table 1, and the full inflow hydrographs are shown on Figure 9. The table and figure also includes the November 2018 event information (USGS, 2019), which is used as a model calibration event.

*Table 1: Peak Boundary Condition Inflows*

Flood Event	Peak Inflow (cfs)
100-year	286,000
50-year	239,400
10-year	147,600
Nov 2018 <sup>a</sup>	62,100

Notes: <sup>a</sup>(USGS, 2019)



*Figure 9: Upstream Inflow Boundaries*

## Numerical Calculations

All hydraulic analysis calculations are performed within the HEC-RAS Version 4.1 (USACE, 2010). The unsteady flow analysis parameters such as start/end time, computational interval, and hydrograph output interval were not



modified i.e., the parameters used are identical to the parameters for the current effective model provided by FEMA.

## Calculation Output

The HEC-RAS Output Tables are provided in Appendix E. The electronic input and output files for all hydraulic models are provided in Appendix B.

## Results

Table 2 summarizes the results of the current effective and duplicate effective hydraulic model runs for the 100-year, 50-year, and 10-year flood events. As shown in the table, the results from the two models are identical.

*Table 2: Comparison of Current Effective and Duplicate Effective Model Water Surface Elevations; 100-year, 50-year, and 10-year Flood Events*

Cross Section/ River Station	100-year Event			50-year Event			10-year Event		
	W.S. Elev (ft NAVD88)			W.S. Elev (ft NAVD88)			W.S. Elev (ft NAVD88)		
	Current	Duplicate	Change	Current	Duplicate	Change	Current	Duplicate	Change
288770	153.97	153.97	0.0	150.86	150.86	0.0	144.37	144.37	0.0
287472	153.85	153.85	0.0	150.61	150.61	0.0	143.69	143.69	0.0
286338	153.60	153.60	0.0	150.36	150.36	0.0	143.39	143.39	0.0
286106	153.36	153.36	0.0	150.11	150.11	0.0	143.14	143.14	0.0
284778	153.02	153.02	0.0	149.81	149.81	0.0	142.92	142.92	0.0
284565 <sup>a</sup>	151.27	151.27	0.0	148.10	148.10	0.0	141.24	141.24	0.0
283052 <sup>b</sup>	150.62	150.62	0.0	147.46	147.46	0.0	140.70	140.70	0.0
281647 <sup>c</sup>	150.19	150.19	0.0	147.08	147.08	0.0	140.44	140.44	0.0
281423	149.95	149.95	0.0	146.88	146.88	0.0	140.31	140.31	0.0
279961	149.29	149.29	0.0	146.28	146.28	0.0	139.90	139.90	0.0
279605	149.18	149.18	0.0	146.18	146.18	0.0	139.81	139.81	0.0
278919	149.03	149.03	0.0	146.03	146.03	0.0	139.70	139.70	0.0

Notes:

- a. Located downstream of Gervais Street bridge
- b. Cross section 'P' on Richland County FIRM (FEMA, 2017) and cross section 'M' on Lexington County FIRM (FEMA, 2018)
- c. Located upstream of Blossom Street bridge

Table 3 summarizes the results of the duplicate effective and corrected effective hydraulic model runs for the 100-year, 50-year, and 10-year flood events. For the 100-year and 50-year flood events, the corrected model results are approximately 0.4ft higher in the vicinity of the Gervais Street bridge and further upstream. Throughout the project area towards the Blossom Street bridge and further downstream, the corrected model results are typically less than 0.05ft higher for the 100-year and 50-year flood events.

For the 10-year flood event, the corrected model results are approximately 0.1ft lower in the vicinity of the Gervais Street bridge and further upstream. Throughout the project area towards the Blossom Street bridge and further downstream, the corrected model results are approximately 0.6ft lower for the 10-year flood event.



*Table 3: Comparison of Duplicate Effective and Corrected Effective Model Water Surface Elevations; 100-year, 50-year, and 10-year Flood Events*

Cross Section/ River Station	100-year Event			50-year Event			10-year Event		
	W.S. Elev (ft NAVD88)			W.S. Elev (ft NAVD88)			W.S. Elev (ft NAVD88)		
	Duplicate	Corrected	Change <sup>a</sup>	Duplicate	Corrected	Change <sup>a</sup>	Duplicate	Corrected	Change <sup>a</sup>
288770	153.97	154.37	0.4	150.86	151.24	0.4	144.37	144.31	-0.1
287472	153.85	154.28	0.4	150.61	151.01	0.4	143.69	143.61	-0.1
286338	153.60	154.04	0.4	150.36	150.78	0.4	143.39	143.31	-0.1
286106	153.36	153.80	0.4	150.11	150.54	0.4	143.14	143.06	-0.1
284778	153.02	153.47	0.5	149.81	150.23	0.4	142.92	142.83	-0.1
284565 <sup>b</sup>	151.27	151.72	0.5	148.10	148.53	0.4	141.24	141.14	-0.1
283052 <sup>c</sup>	150.62	150.65	0.0	147.46	147.51	0.1	140.70	140.26	-0.4
281647 <sup>d</sup>	150.19	150.20	0.0	147.08	147.09	0.0	140.44	139.89	-0.6
281423	149.95	149.96	0.0	146.88	146.89	0.0	140.31	139.75	-0.6
279961	149.29	149.30	0.0	146.28	146.29	0.0	139.90	139.30	-0.6
279605	149.18	149.19	0.0	146.18	146.18	0.0	139.81	139.22	-0.6
278919	149.03	149.04	0.0	146.03	146.03	0.0	139.70	139.09	-0.6

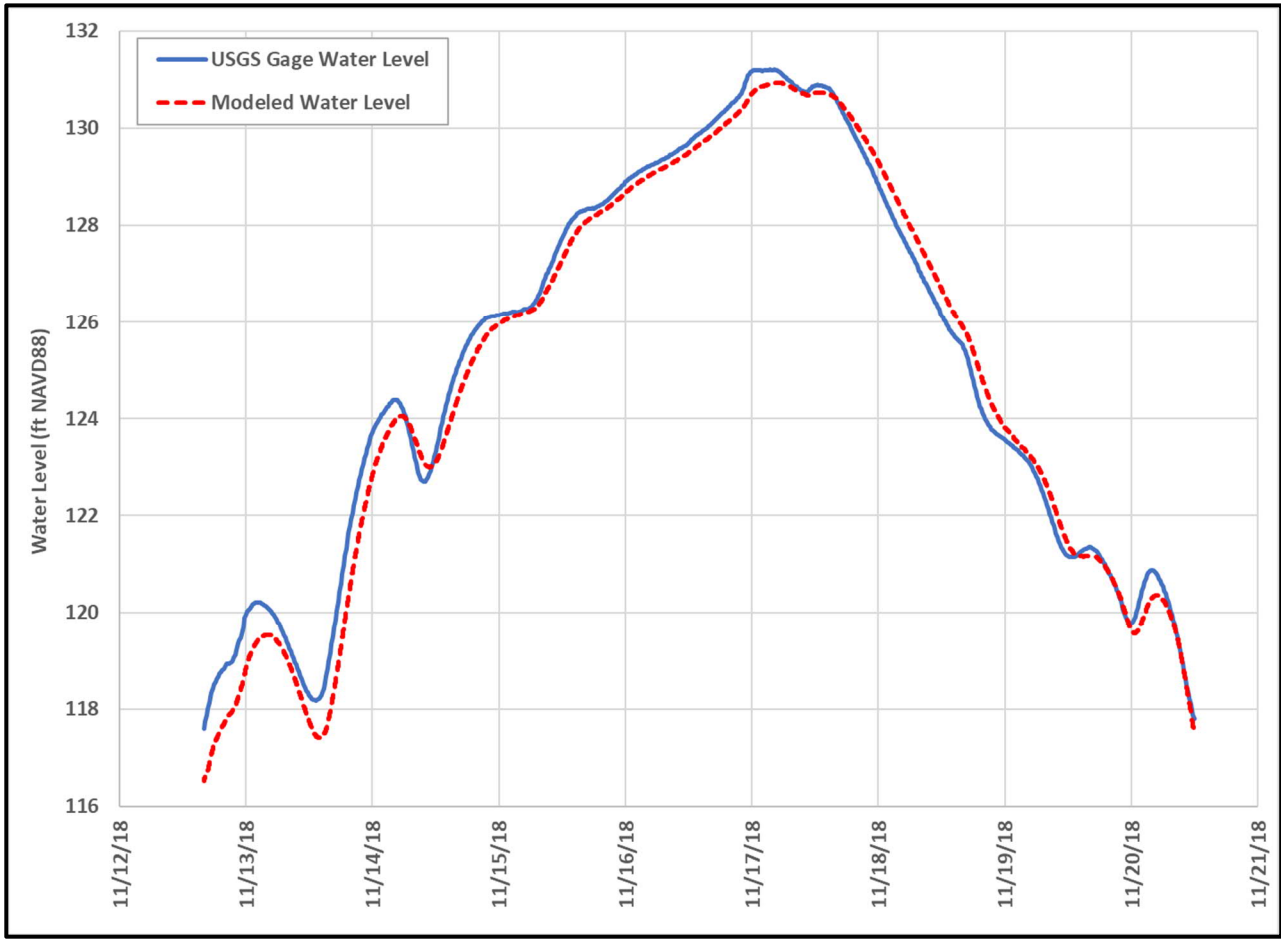
Notes:

- a. 'Change' is calculated by subtracting 'Proposed' from 'Corrected' and rounding to one decimal place
- b. Located downstream of Gervais Street bridge
- c. Cross section 'P' on Richland County FIRM (FEMA, 2017) and cross section 'M' on Lexington County FIRM (FEMA, 2018)
- d. Located upstream of Blossom Street bridge

The localized changes to peak water surface elevations is the result of the updated topographic data and additional cross sections that provide a more accurate representation of the channel conveyance and floodplain storage throughout the project area. However, the changes do not result in any changes to the 100-year flood level published on the FEMA FIRM (FEMA, 2017/2018) at River Station 283052.

The United States Geological Survey (USGS) gage 02169500 is located on the Congaree River corrected on the west bank opposite the locations of the proposed cofferdams. The USGS gage data (USGS, 2019) was reviewed to select a high flow event that could be simulated using to model, to determine how accurately the HEC-RAS model results matched the gage data. A high flow event from November 2018 was selected for calibration, with a peak flow of 62,100 cfs. A comparison of the corrected effective HEC-RAS results and the USGS gage data from this event is shown in Figure 10.

Figure 10 shows good correlation between the corrected effective model results and the USGS gage data. The peak water surface elevation predicted by the corrected effective HEC-RAS model is 0.26ft lower than the value reported by the USGS gage.



*Figure 10: Comparison of HEC-RAS Corrected Effective Model Results and USGS Gage Data for November 2018 High Flow Event*

The corrected model is used as a baseline for comparison with the proposed models, which have been developed to represent the conditions incorporating the Area-1 and Area 2 cofferdam structures (each area is represented in a separate model, as the work will be completed using a phased approach).

Table 4 summarizes the results of the corrected effective and proposed Area-1 hydraulic model runs for the 100-year, 50-year, and 10-year flood events. The results demonstrate that the impact of the proposed Area-1 cofferdam structure is relatively consistent for the 100-year, 50-year, and 10-year flood events. The results typically show an increase of approximately 0.04ft upstream of the Area-1 cofferdam, and an increase of 0.01 to 0.03ft in the middle of the structure. At the upstream and downstream cofferdam extents, the results show there is reduction of approximately 0.1ft, and downstream of the structure there is no change to the peak water surface elevations.





Table 4: Comparison of Corrected Effective and Proposed Area-1 Model Water Surface Elevations; 100-year, 50-year, and 10-year Flood Events

Cross Section/ River Station	100-year Event			50-year Event			10-year Event		
	W.S. Elev (ft NAVD88)			W.S. Elev (ft NAVD88)			W.S. Elev (ft NAVD88)		
	Corrected	Proposed	Change <sup>a</sup>	Corrected	Proposed	Change <sup>a</sup>	Corrected	Proposed	Change <sup>a</sup>
288770	154.374	154.412	0.0	151.235	151.273	0.0	144.313	144.341	0.0
287472	154.278	154.318	0.0	151.011	151.052	0.0	143.609	143.644	0.0
286338	154.040	154.081	0.0	150.776	150.819	0.0	143.308	143.345	0.0
286106	153.804	153.846	0.0	150.535	150.578	0.0	143.055	143.094	0.0
284778	153.465	153.508	0.0	150.234	150.278	0.0	142.829	142.869	0.0
284565 <sup>b</sup>	151.716	151.759	0.0	148.525	148.569	0.0	141.140	141.180	0.0
284431	151.610	151.653	0.0	148.398	148.443	0.0	140.999	141.039	0.0
284408 <sup>c</sup>	151.479	151.423	-0.1	148.301	148.248	-0.1	140.947	140.888	-0.1
284395 <sup>c</sup>	151.453	151.393	-0.1	148.279	148.222	-0.1	140.937	140.876	-0.1
284372 <sup>c</sup>	151.418	151.449	0.0	148.248	148.282	0.0	140.922	140.953	0.0
284267 <sup>c</sup>	151.234	151.251	0.0	148.092	148.113	0.0	140.827	140.847	0.0
284060 <sup>c</sup>	151.035	151.048	0.0	147.912	147.928	0.0	140.691	140.705	0.0
283820 <sup>c</sup>	150.852	150.850	0.0	147.724	147.724	0.0	140.537	140.535	0.0
283636 <sup>c</sup>	150.781	150.641	-0.1	147.654	147.503	-0.2	140.467	140.306	-0.2
283611 <sup>c</sup>	150.683	150.546	-0.1	147.566	147.423	-0.1	140.406	140.255	-0.2
283601 <sup>c</sup>	150.739	150.618	-0.1	147.593	147.459	-0.1	140.392	140.236	-0.2
283574	150.696	150.696	0.0	147.536	147.536	0.0	140.323	140.323	0.0
283490	150.711	150.711	0.0	147.558	147.558	0.0	140.268	140.268	0.0
283342	150.727	150.727	0.0	147.570	147.570	0.0	140.233	140.233	0.0
283203	150.668	150.668	0.0	147.525	147.525	0.0	140.257	140.257	0.0
283179	150.658	150.658	0.0	147.518	147.518	0.0	140.206	140.206	0.0
283169	150.665	150.665	0.0	147.524	147.524	0.0	140.211	140.211	0.0
283139	150.642	150.642	0.0	147.500	147.500	0.0	140.204	140.204	0.0
283093	150.688	150.688	0.0	147.533	147.533	0.0	140.206	140.206	0.0
283052 <sup>d</sup>	150.652	150.652	0.0	147.506	147.506	0.0	140.257	140.257	0.0
283001	150.753	150.753	0.0	147.594	147.594	0.0	140.320	140.320	0.0
282937	150.706	150.706	0.0	147.552	147.552	0.0	140.110	140.110	0.0
282912	150.679	150.679	0.0	147.525	147.525	0.0	140.209	140.209	0.0
282902	150.676	150.676	0.0	147.521	147.521	0.0	140.194	140.194	0.0
282874	150.641	150.641	0.0	147.498	147.498	0.0	140.157	140.157	0.0
282707	150.511	150.511	0.0	147.358	147.358	0.0	139.929	139.929	0.0
282424	150.448	150.448	0.0	147.299	147.299	0.0	139.966	139.966	0.0
282071	150.344	150.344	0.0	147.204	147.204	0.0	139.950	139.950	0.0
281647 <sup>e</sup>	150.199	150.199	0.0	147.087	147.087	0.0	139.888	139.888	0.0
281423	149.960	149.960	0.0	146.886	146.886	0.0	139.748	139.748	0.0
279961	149.301	149.301	0.0	146.289	146.289	0.0	139.302	139.302	0.0
279605	149.191	149.191	0.0	146.183	146.183	0.0	139.215	139.215	0.0
278919	149.039	149.039	0.0	146.033	146.033	0.0	139.088	139.088	0.0

Notes:

- a. 'Change' is calculated by subtracting 'Proposed' from 'Corrected' and rounding to one decimal place
- b. Located downstream of Gervais Street bridge
- c. Area-1 cofferdam
- d. Cross section 'P' on Richland County FIRM (FEMA, 2017) and cross section 'M' on Lexington County FIRM (FEMA, 2018)
- e. Located upstream of Blossom Street bridge

Table 5 summarizes the results of the corrected effective and proposed Area-2 hydraulic model runs for the 100-year, 50-year, and 10-year flood events. The results demonstrate that the impact of the proposed Area-2 cofferdam structure is relatively consistent for the 100-year, 50-year, and 10-year flood events. The results



typically show an increase of approximately 0.02ft upstream of the Area-2 cofferdam. From the upstream to the downstream cofferdam extents, the results show there is reduction typically between 0.01 and 0.1ft, with reductions up to 0.2ft at the downstream extent. Downstream of the structure there is no change to the peak water surface elevations.



Table 5: Comparison of Corrected Effective and Proposed Area-2 Model Water Surface Elevations; 100-year, 50-year, and 10-year Flood Events

Cross Section/ River Station	100-year Event			50-year Event			10-year Event		
	W.S. Elev (ft NAVD88)			W.S. Elev (ft NAVD88)			W.S. Elev (ft NAVD88)		
	Corrected	Proposed	Change <sup>a</sup>	Corrected	Proposed	Change <sup>a</sup>	Corrected	Proposed	Change <sup>a</sup>
288770	154.374	154.389	0.0	151.235	151.251	0.0	144.313	144.320	0.0
287472	154.278	154.294	0.0	151.011	151.028	0.0	143.609	143.619	0.0
286338	154.040	154.057	0.0	150.776	150.794	0.0	143.308	143.318	0.0
286106	153.804	153.821	0.0	150.535	150.553	0.0	143.055	143.066	0.0
284778	153.465	153.482	0.0	150.234	150.253	0.0	142.829	142.840	0.0
284565 <sup>b</sup>	151.716	151.733	0.0	148.525	148.544	0.0	141.140	141.151	0.0
284431	151.610	151.627	0.0	148.398	148.416	0.0	140.999	141.009	0.0
284408	151.479	151.496	0.0	148.301	148.319	0.0	140.947	140.958	0.0
284395	151.453	151.470	0.0	148.279	148.297	0.0	140.937	140.947	0.0
284372	151.418	151.436	0.0	148.248	148.267	0.0	140.922	140.933	0.0
284267	151.234	151.252	0.0	148.092	148.111	0.0	140.827	140.838	0.0
284060	151.035	151.053	0.0	147.912	147.931	0.0	140.691	140.702	0.0
283820	150.852	150.870	0.0	147.724	147.744	0.0	140.537	140.549	0.0
283636	150.781	150.800	0.0	147.654	147.675	0.0	140.467	140.479	0.0
283611	150.683	150.702	0.0	147.566	147.586	0.0	140.406	140.418	0.0
283601	150.739	150.758	0.0	147.593	147.613	0.0	140.392	140.403	0.0
283574	150.696	150.716	0.0	147.536	147.557	0.0	140.323	140.334	0.0
283490	150.711	150.730	0.0	147.558	147.578	0.0	140.268	140.279	0.0
283342	150.727	150.747	0.0	147.570	147.590	0.0	140.233	140.245	0.0
283203	150.668	150.686	0.0	147.525	147.544	0.0	140.257	140.268	0.0
283179 <sup>c</sup>	150.658	150.594	-0.1	147.518	147.452	-0.1	140.206	140.085	-0.1
283169 <sup>c</sup>	150.665	150.597	-0.1	147.524	147.453	-0.1	140.211	140.081	-0.1
283139 <sup>c</sup>	150.642	150.639	0.0	147.500	147.498	0.0	140.204	140.182	0.0
283093 <sup>c</sup>	150.688	150.685	0.0	147.533	147.529	0.0	140.206	140.183	0.0
283052 <sup>c+d</sup>	150.652	150.651	0.0	147.506	147.503	0.0	140.257	140.242	0.0
283001 <sup>c</sup>	150.753	150.757	0.0	147.594	147.598	0.0	140.320	140.312	0.0
282937 <sup>c</sup>	150.706	150.636	-0.1	147.552	147.477	-0.1	140.110	139.906	-0.2
282912 <sup>c</sup>	150.679	150.605	-0.1	147.525	147.447	-0.1	140.209	140.046	-0.2
282902 <sup>c</sup>	150.676	150.598	-0.1	147.521	147.439	-0.1	140.194	139.945	-0.2
282874	150.641	150.641	0.0	147.498	147.498	0.0	140.157	140.157	0.0
282707	150.511	150.511	0.0	147.358	147.358	0.0	139.929	139.929	0.0
282424	150.448	150.448	0.0	147.299	147.299	0.0	139.966	139.966	0.0
282071	150.344	150.344	0.0	147.204	147.204	0.0	139.950	139.950	0.0
281647 <sup>e</sup>	150.199	150.199	0.0	147.087	147.087	0.0	139.888	139.888	0.0
281423	149.960	149.960	0.0	146.886	146.886	0.0	139.748	139.748	0.0
279961	149.301	149.301	0.0	146.289	146.289	0.0	139.302	139.302	0.0
279605	149.191	149.191	0.0	146.183	146.183	0.0	139.215	139.214	0.0
278919	149.039	149.039	0.0	146.033	146.033	0.0	139.088	139.088	0.0

Notes:

- a. 'Change' is calculated by subtracting 'Proposed' from 'Corrected' and rounding to one decimal place
- b. Located downstream of Gervais Street bridge
- c. Area-2 cofferdam
- d. Cross section 'P' on Richland County FIRM (FEMA, 2017) and cross section 'M' on Lexington County FIRM (FEMA, 2018)
- e. Located upstream of Blossom Street bridge



## Conclusion/Summary

The results in Table 4 and Table 5 show the addition of the proposed Area-1 and Area-2 cofferdam structures results in maximum increases in water surface elevations of 0.043, 0.045, and 0.040 ft for the 100-year, 50-year, and 10-year flood events, respectively. These increases are negligible and are considered to be within the accuracy limits of the hydraulic model. It is standard industry practice to report hydraulic model results to the nearest tenth of a foot. Reporting model results or changes in model results to a greater number of decimal places implies a level of accuracy that is simply not practical to achieve.

When the changes in peak water surface elevations are reported to the nearest tenth of a foot, all increases are rounded down to zero, therefore the proposed Area-1 and Area-2 cofferdam structures will result in no-rise in the 100-year BFE within the extents of the model.

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## APPENDICES



## Appendix A: FEMA Procedures for “No-Rise” Certification for Proposed Developments in the Regulatory Floodway



# FEMA

US Department of Homeland Security  
Region X  
130 228<sup>th</sup> Street, SW  
Bothell, WA 98021

## ***Procedures for “No-Rise” Certification*** **For Proposed Developments in the Regulatory Floodway**

Section 60.3 (d) (3) of the National Flood Insurance Program (NFIP) regulations states that a community shall "prohibit encroachments, including fill, new construction, substantial improvements and other development within the adopted regulatory floodway unless it has been demonstrated through hydrologic and hydraulic analyses performed in accordance with standard engineering practice that the proposed encroachment would not result in any increase in flood levels within the community during the occurrence of the base (100-year) flood discharge."

Prior to issuing any building, grading or development permits involving activities in a regulatory floodway the community must obtain a certification stating the proposed development will not impact the pre-project base flood elevations, floodway elevations, or floodway data widths. The certification should be obtained from the applicant and be signed and sealed by a professional engineer.

The engineering or "no-rise" certification must be supported by technical data.

The supporting technical data should be based upon hydraulic analyses that utilize the same model used to prepare the effective Flood Insurance Study (FIS) report and Flood Insurance Rate Map (FIRM) unless it is demonstrated that the 'effective' hydraulic model is unavailable or its use is inappropriate. If an alternative hydraulic model is used, the new model must be calibrated to reproduce the FIS profiles within 0.5 feet. Hydraulic model used in the analysis must be on FEMA's accepted models list, or documentation must be provided showing the model meets the requirements of NFIP regulation 65.6(a)(6).

Although communities are required to review and approve the "no-rise" submittals, they may request, in writing, technical assistance and review from the FEMA regional office. However, if this alternative is chosen, the community must review the technical submittal package and verify that all supporting data, listed in the following paragraphs, are included in the package before forwarding to FEMA.

To support a "no-rise" certification for proposed developments encroaching into the regulatory floodway, a community will require that the following procedures be followed:

1. Current Effective Model: Submit a written request for the effective model for the specified stream and community, identifying the limits of the requested data. A fee will be assessed for providing the data. Data request forms and instructions can be obtained at:

<http://www.fema.gov/national-flood-insurance-program-flood-hazard-mapping/how-order-technical-administrative-support>

or by writing to:

FEMA Engineering Library  
847 S. Pickett Street  
Alexandria, VA 22304  
Phone: 1-877-336-2627  
Facsimile: 1-703-212-4090

2. Duplicate Effective Model: Upon receipt of the effective computer model, the engineer should run the original model to duplicate the output in the effective (FIS).
3. Corrected Effective Model: The model that corrects any errors that occur in the Duplicate Effective model, adds any additional cross sections, or incorporates more detailed topographic information than that used in the current effective model. Floodway limits should be manually set at the new cross-section locations by measuring from the effective FIRM or FBFM. The cumulative reach lengths of the stream should also remain unchanged. The Corrected Effective model must not reflect any man-made physical changes since the date of the effective model.
4. Existing, or Pre-Project Conditions Model: Revise the Duplicate Effective or the Corrected Effective model to reflect any modifications that have occurred within the floodplain since the date of the Effective model but prior to the construction of the project. If no modifications have occurred since the date of the effective model, then the model would be identical to the Duplicate Effective or Corrected Effective model. The results of this



Existing Conditions analysis will indicate the 100-yr elevations at the project site.

5. Proposed, or Post-Project Conditions Model: Modify the Existing Condition or Pre-Project Conditions Model (or Duplicate Effective model or Corrected Effective model, as appropriate) to reflect revised or post-project conditions. The overbank roughness coefficients should remain the same unless a reasonable explanation of how the proposed development will impact Manning's "n" values is included with the supporting data. The results of this analysis will indicate the 100-year elevation for proposed conditions at the project site. These results must indicate NO impact on the 100-year floodway elevations when compared to the Existing Conditions or Pre-Project Conditions model. If an increase results the project will require the submittal of a CLOMR prior to the start of the project.

The "no-rise" supporting data and a copy of the engineering certification must be submitted to and reviewed by the appropriate community official prior to issuing a permit.

The "no-rise" supporting data should include, but may not be limited to:

- 1) Copy of the Duplicate Effective model;
- 2) Copy of the Corrected Effective model;
- 3) Existing conditions, or Pre-Project conditions model
- 4) Proposed conditions or Post-Project conditions model.
- 5) FIRM and topographic map, showing floodplain and floodway, the additional cross-sections, the site location with the proposed topographic modification superimposed onto the maps, and a copy of the effective FIRM or FBFM showing the current regulatory floodway.
- 6) Documentation clearly stating analysis procedures. All modifications made to the original FIS model to represent revised existing conditions, as well as those made to the revised existing conditions model to represent proposed conditions, should be well documented and submitted with all supporting data.
- 7) Copy of effective Floodway Data Table copied from the (FIS) report.
- 8) Statement defining source of additional cross-section topographic data and supporting information.
- 9) Cross-section plots, of the added cross sections, for revised existing and proposed conditions.

- 10) Certified planimetric (boundary survey) information indicating the location of structures on the property.
- 11) Copy of the source from which input for original FIS model was taken.
- 12) CD with all input and output files.
- 13) Printout of output files from EDIT runs for all three floodway models.

The engineering "no-rise" certification and-supporting technical data must stipulate NO impact on the 100-year flood or floodway elevations at the new cross-sections and at all existing cross-sections anywhere in the model. Therefore, the revised computer model should be run for a sufficient distance (usually one mile, depending on hydraulic slope of the stream) upstream and downstream of the development site to insure proper "no-rise" certification.

Attached is a sample "no-rise" certification form that can be completed by a registered professional engineer and supplied to the community along with the supporting technical data when applying for a development permit.

ENGINEERING "NO-RISE" CERTIFICATION

This is to certify that I am a duly qualified engineer licensed to practice in the State of \_\_\_\_\_.

It is to further certify that the attached technical data supports the fact that proposed \_\_\_\_\_ will

*(Name of Development)*

not impact the 100-year flood elevations, floodway elevations and floodway widths on \_\_\_\_\_ at published sections

*(Name of Stream)*

in the Flood Insurance Study for \_\_\_\_\_,

*(Name of Community)*

dated \_\_\_\_\_ and will not impact the 100-year flood elevations, floodway elevations, and floodway widths at unpublished cross-sections in the vicinity of the proposed development.

Attached are the following documents that support my findings:

\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

(Date) \_\_\_\_\_

(Signature)

(Title)

\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

(Address)

(Seal)

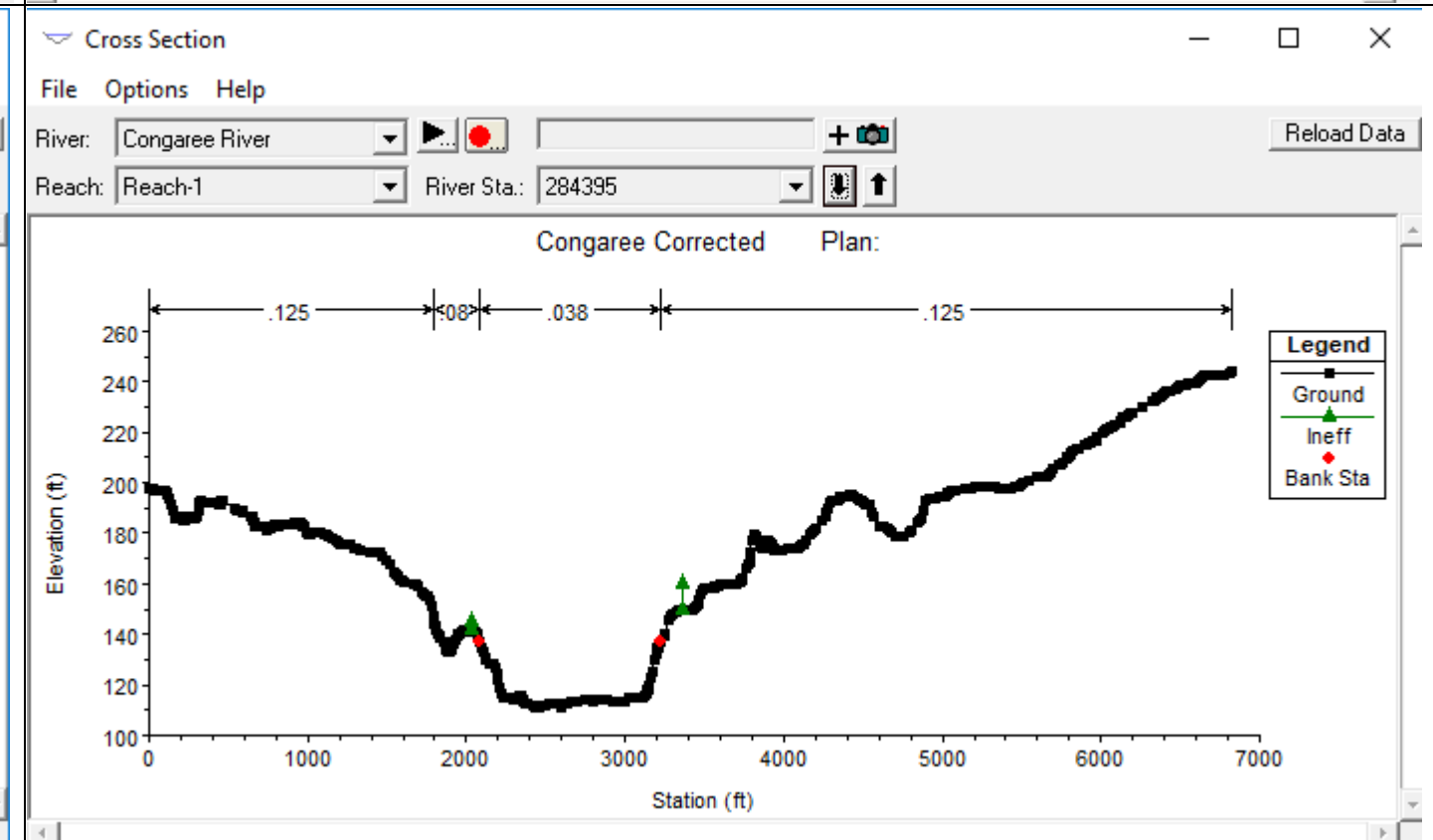
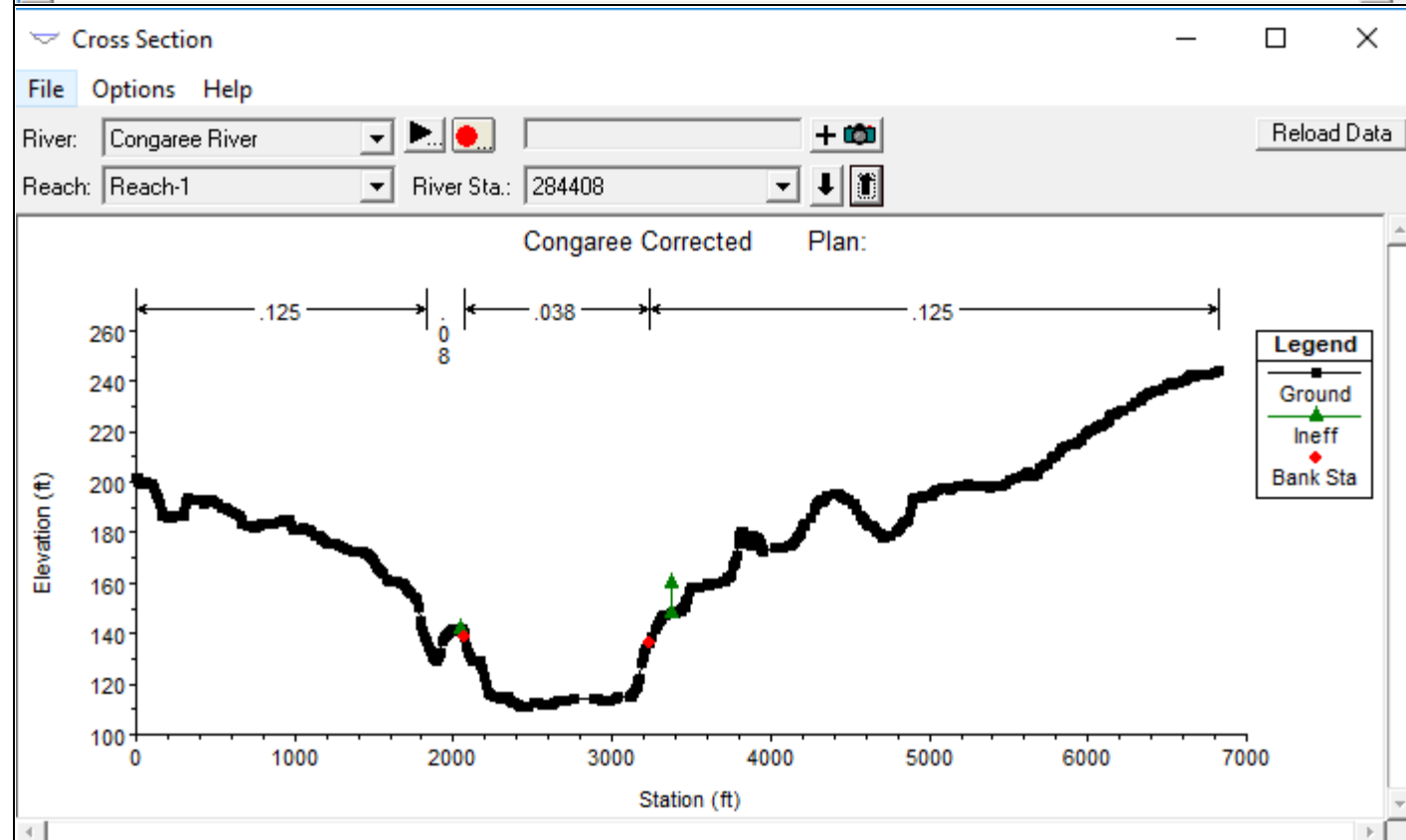
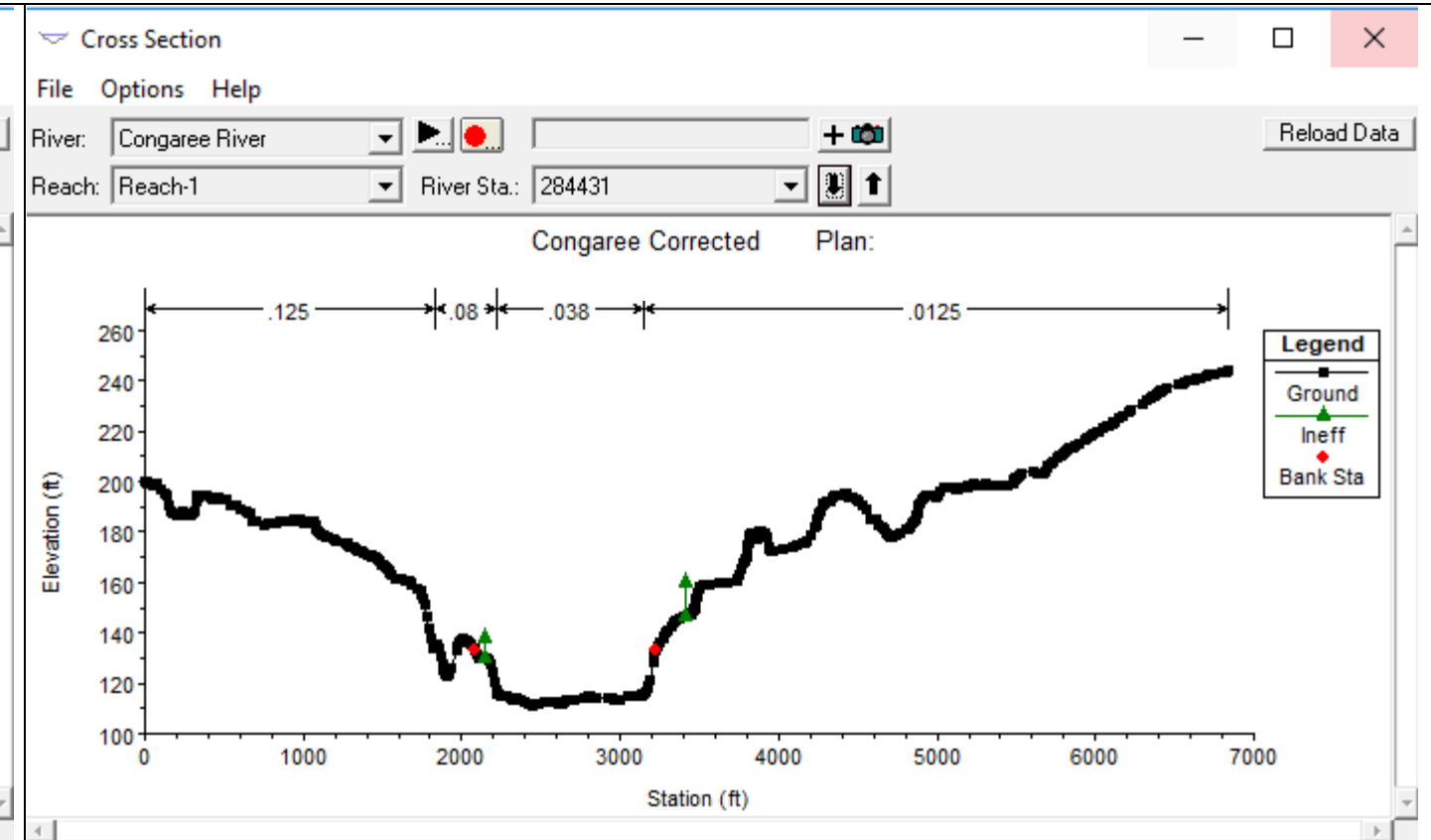
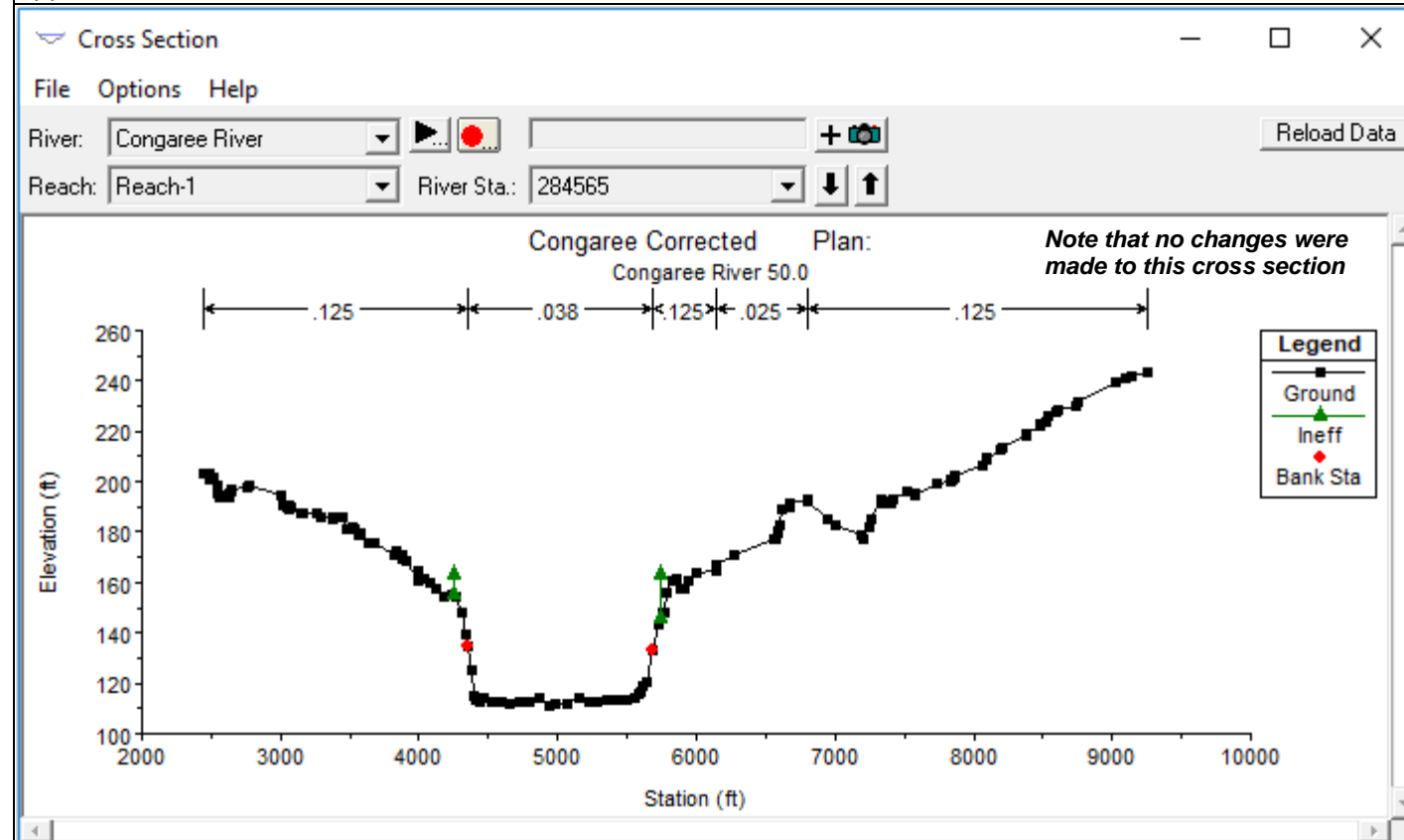


## Appendix B: Electronic Files

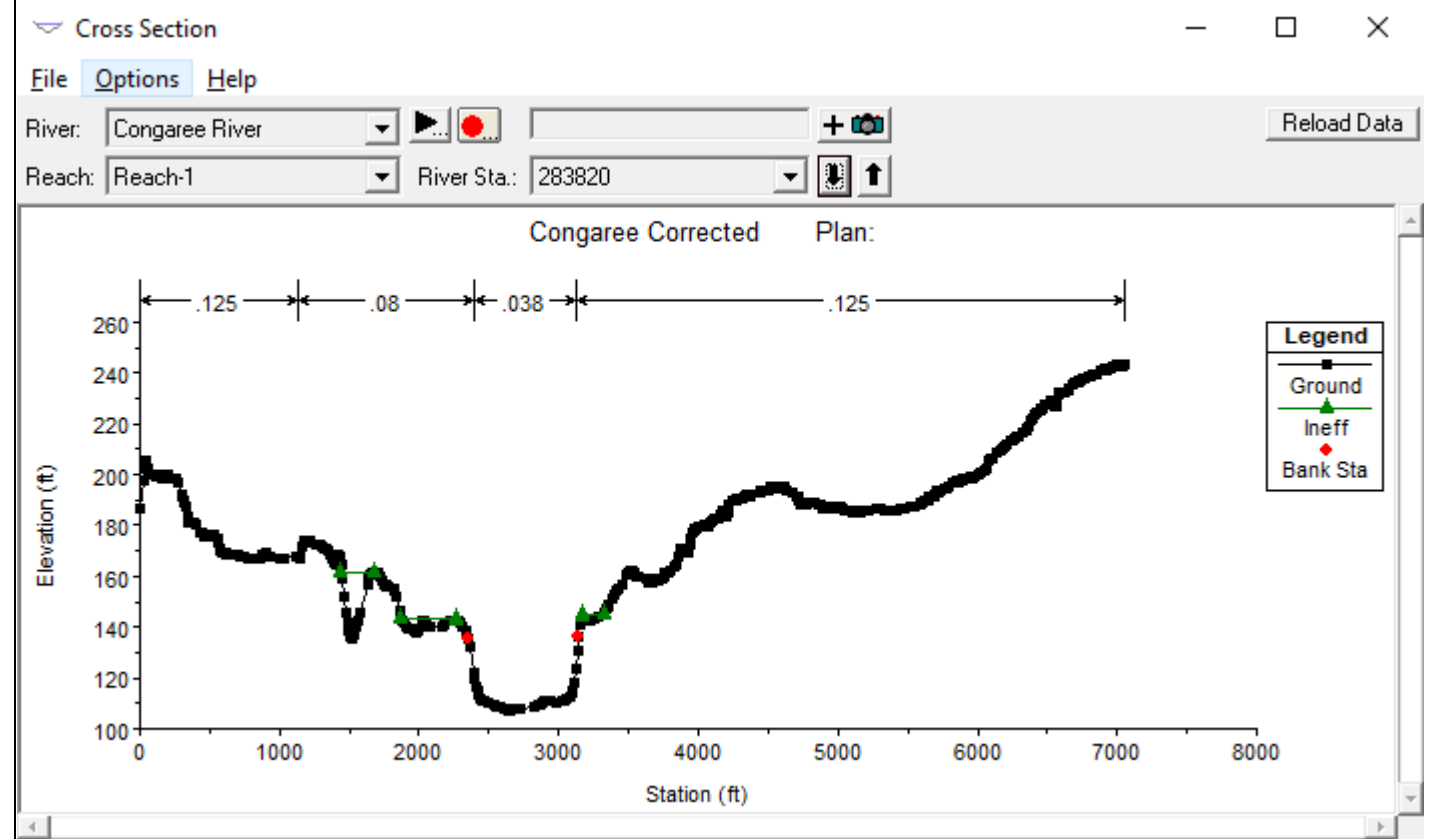
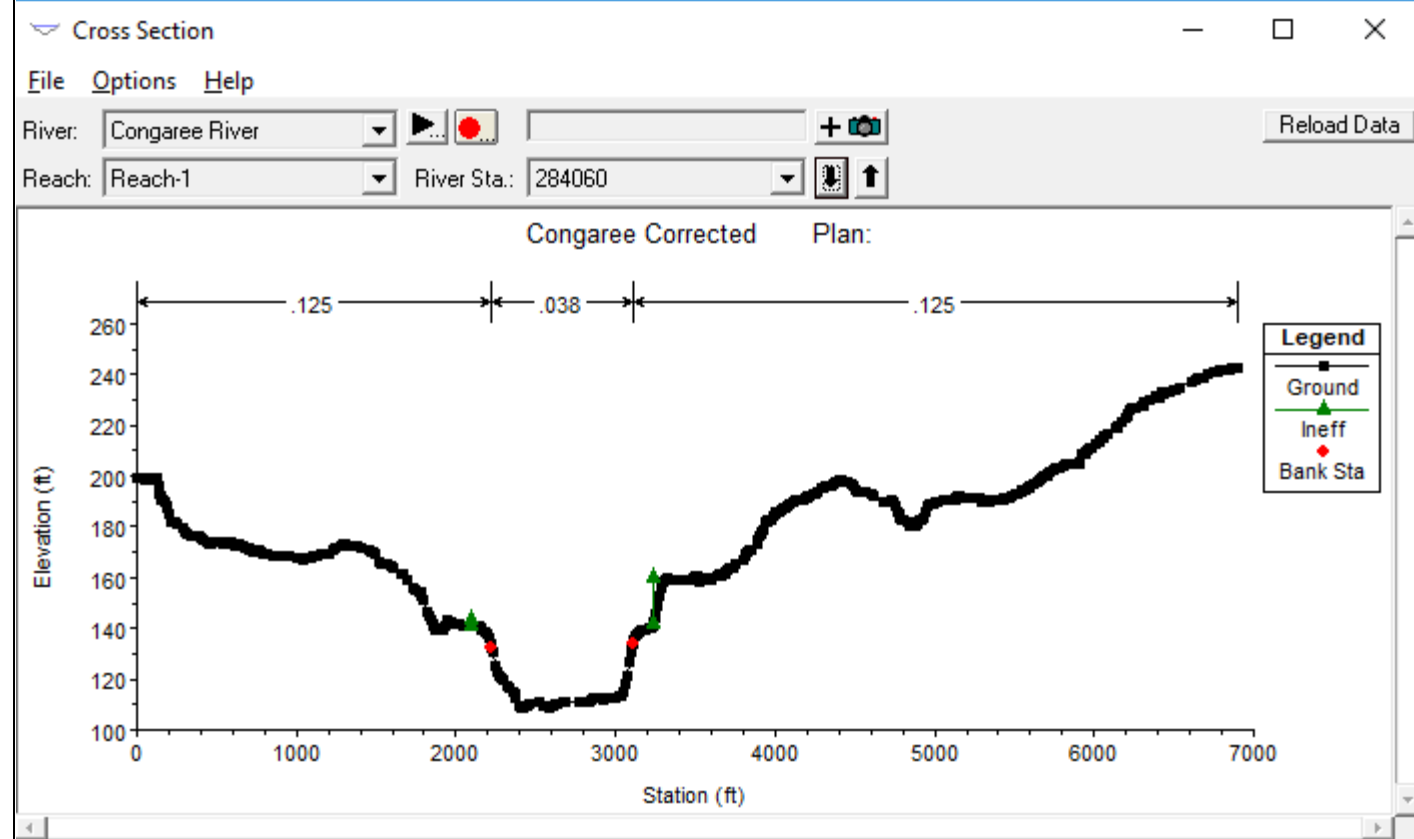
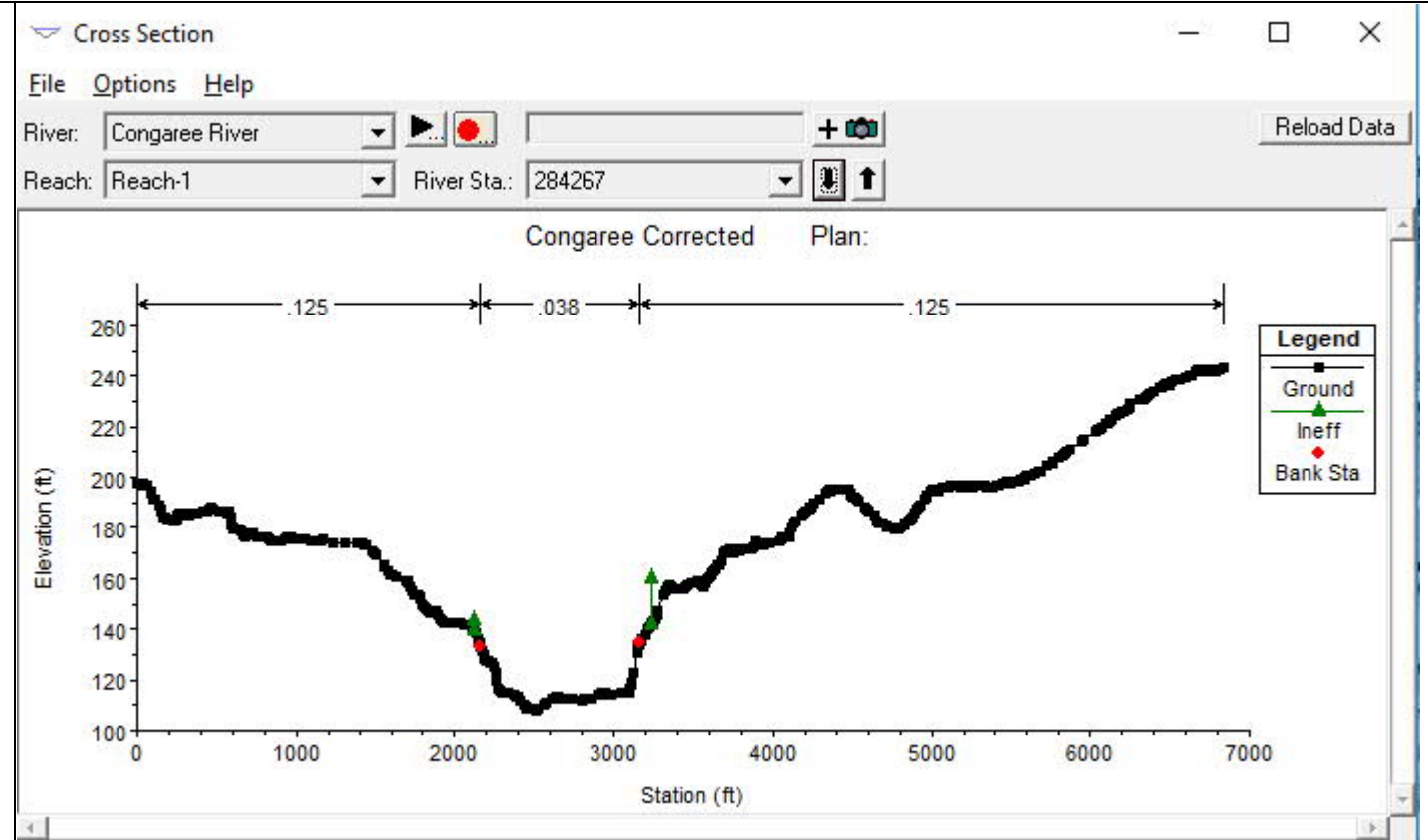
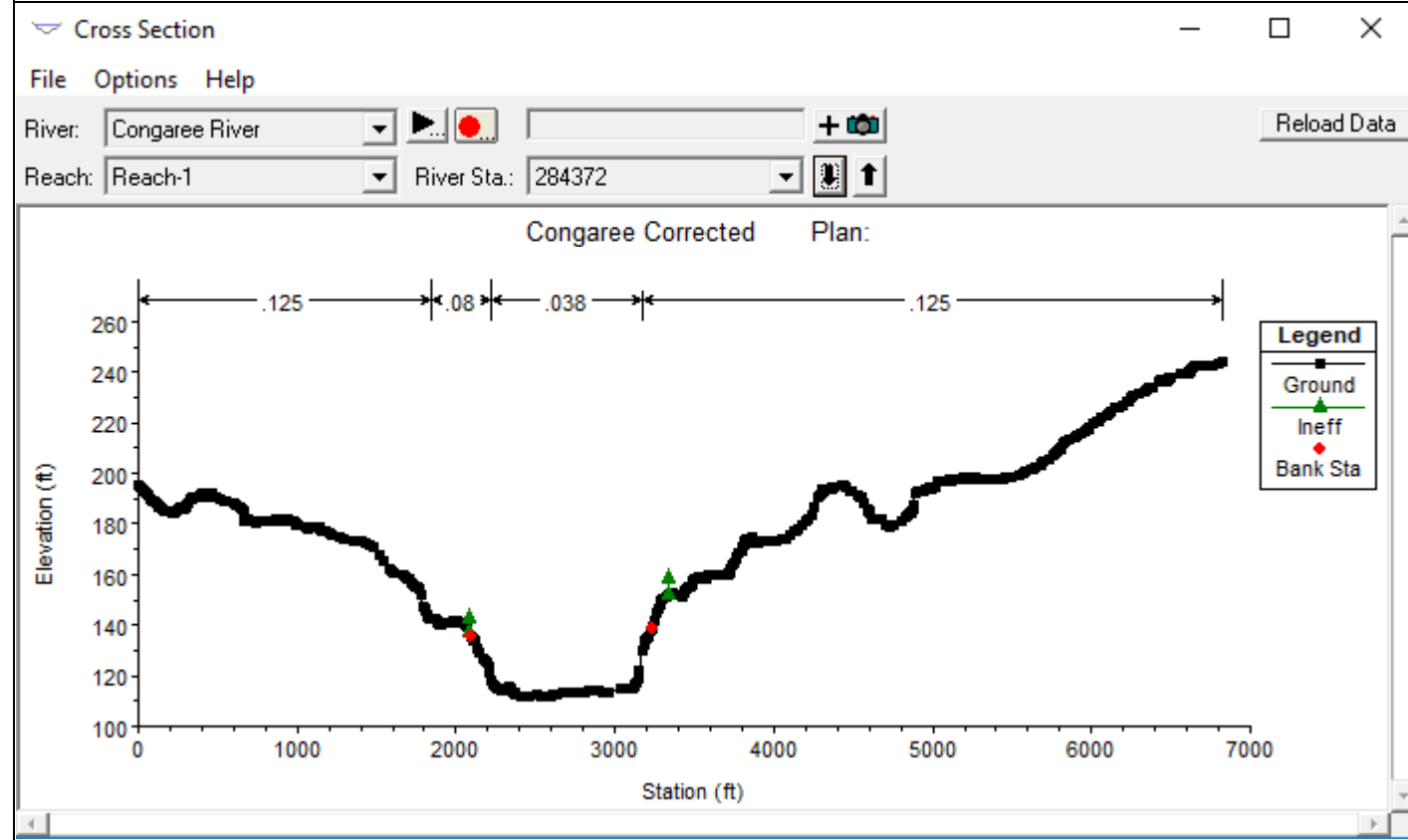


## Appendix C: Corrected Effective Model Cross Sections

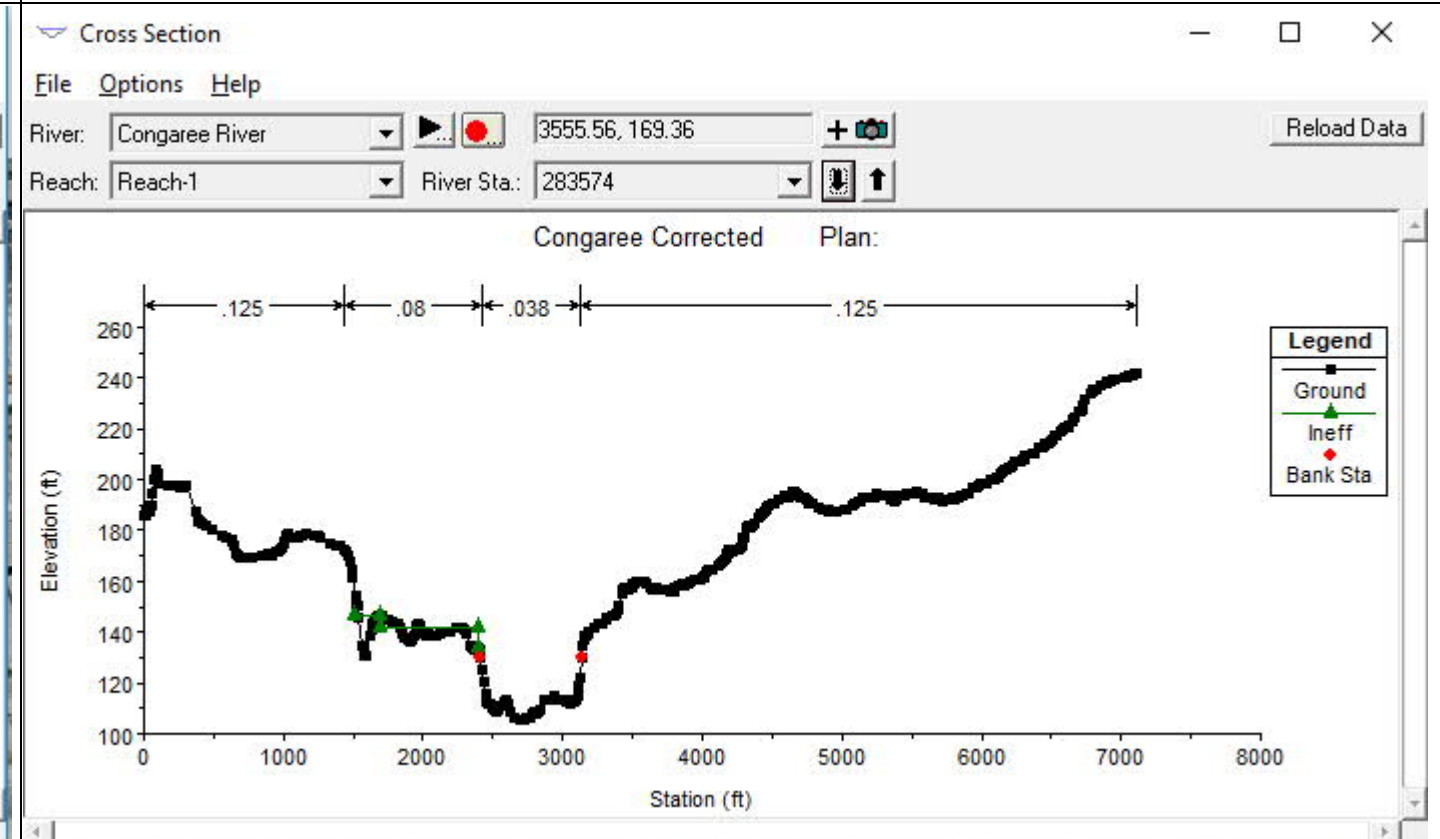
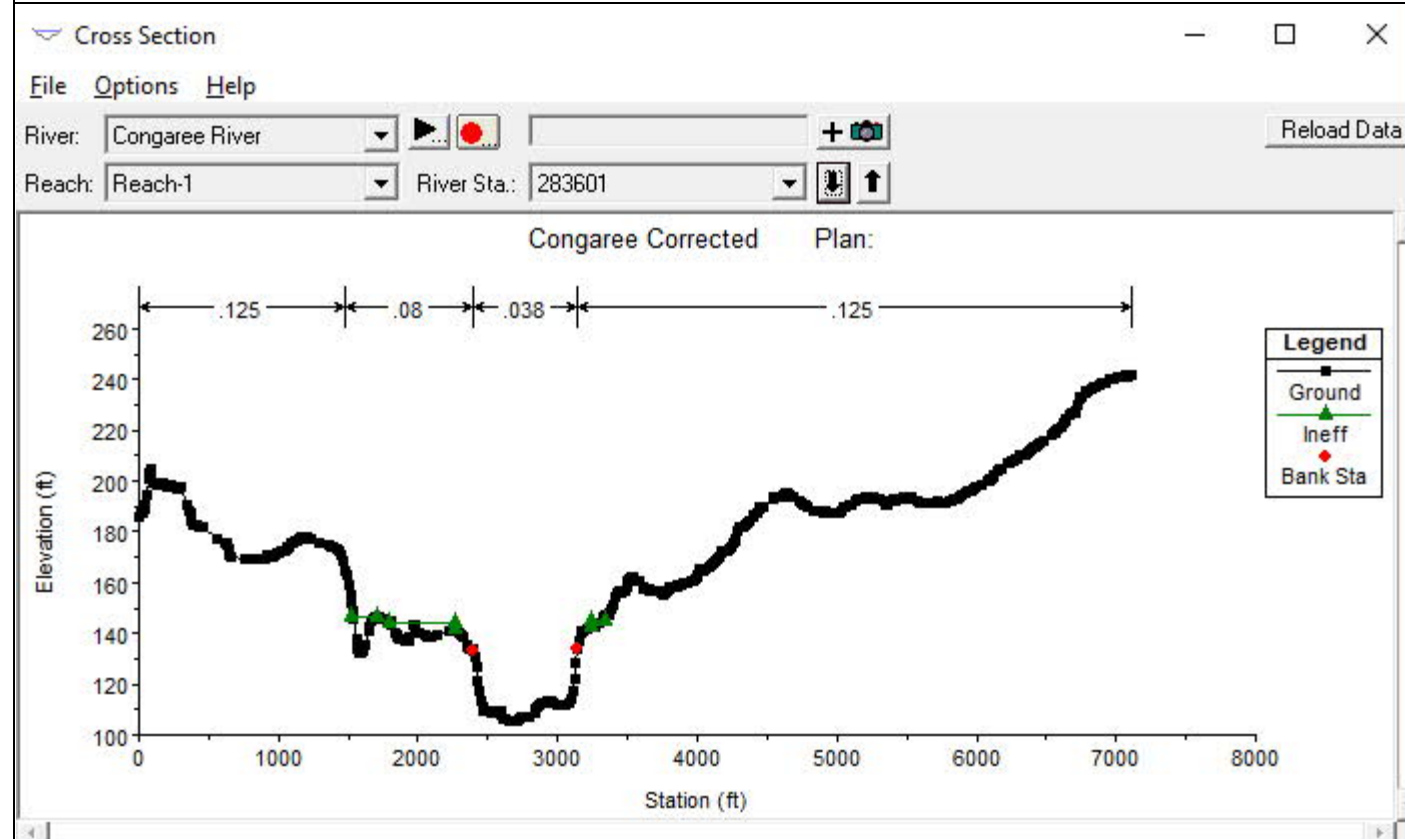
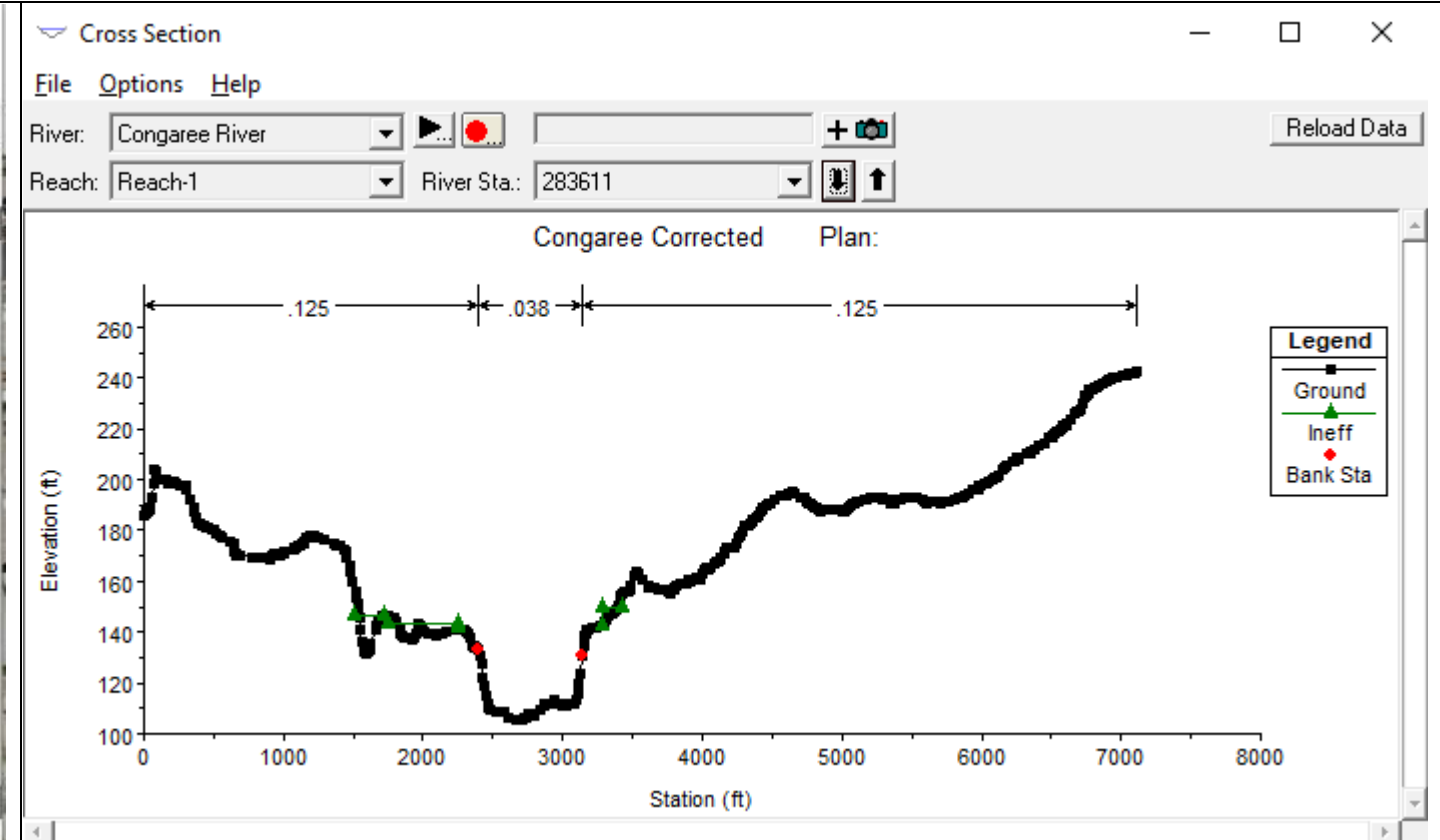
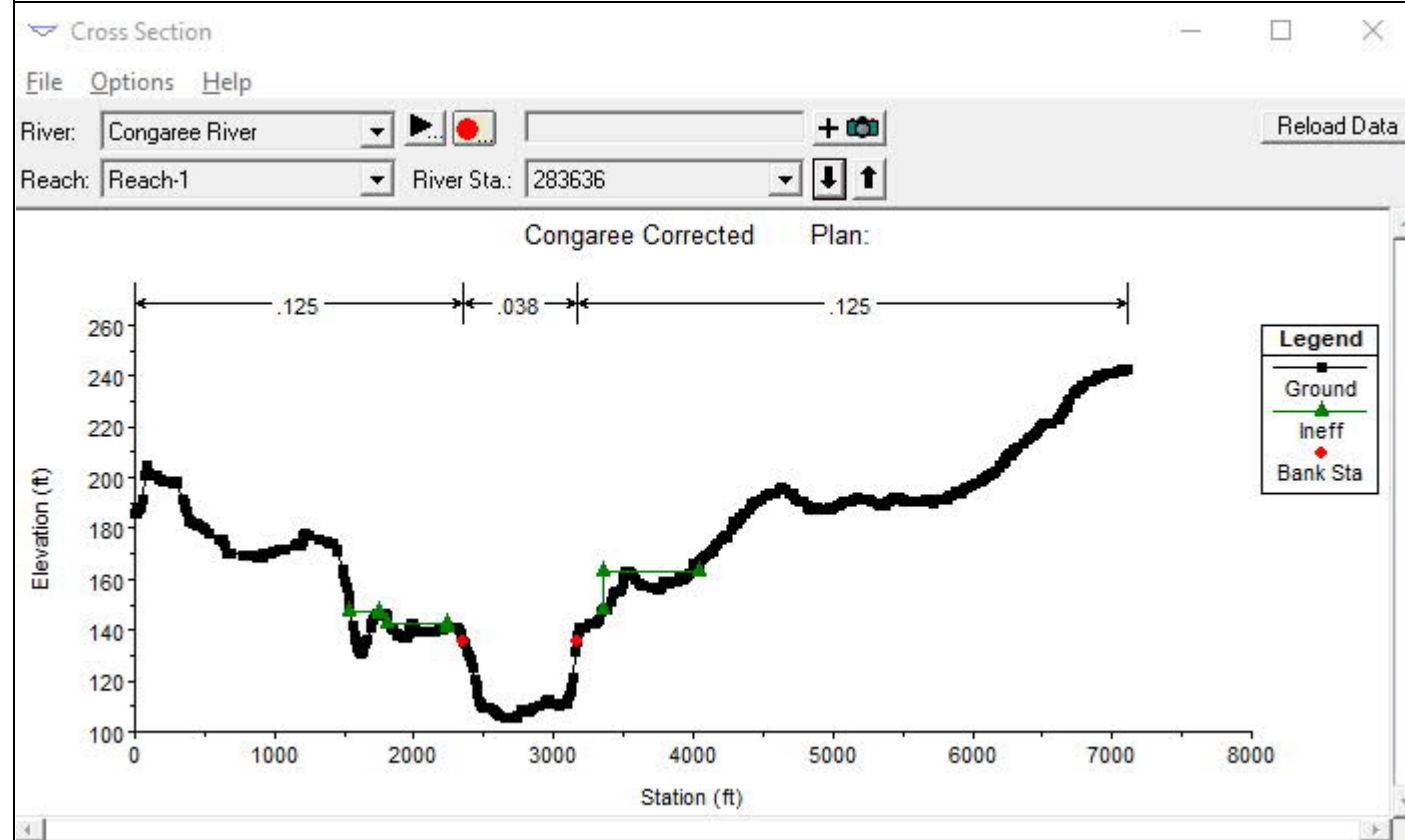
Appendix C: Corrected Effective Model Cross Sections (Sheet 1 of 8)



Appendix C: Corrected Effective Model Cross Sections (Sheet 2 of 8)

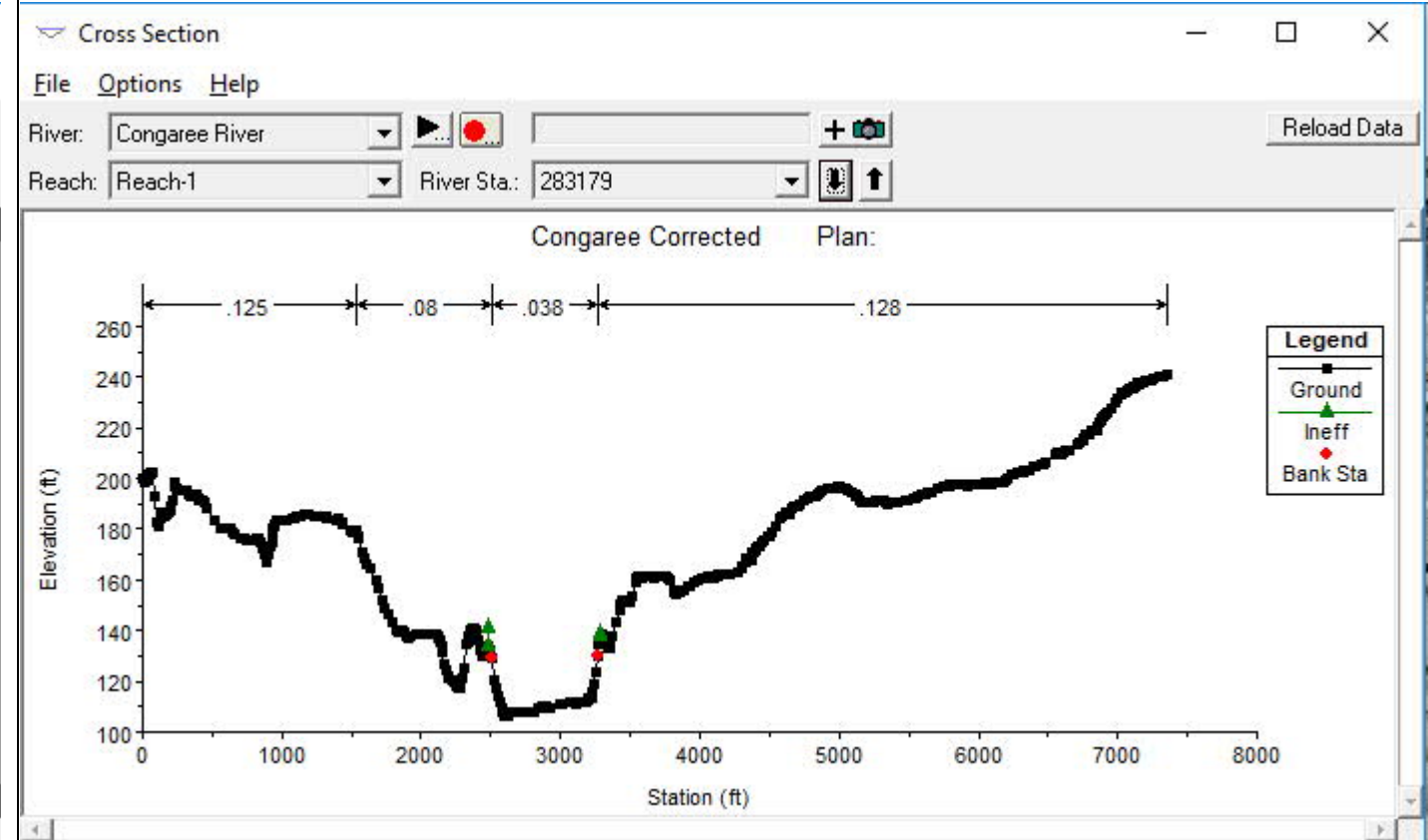
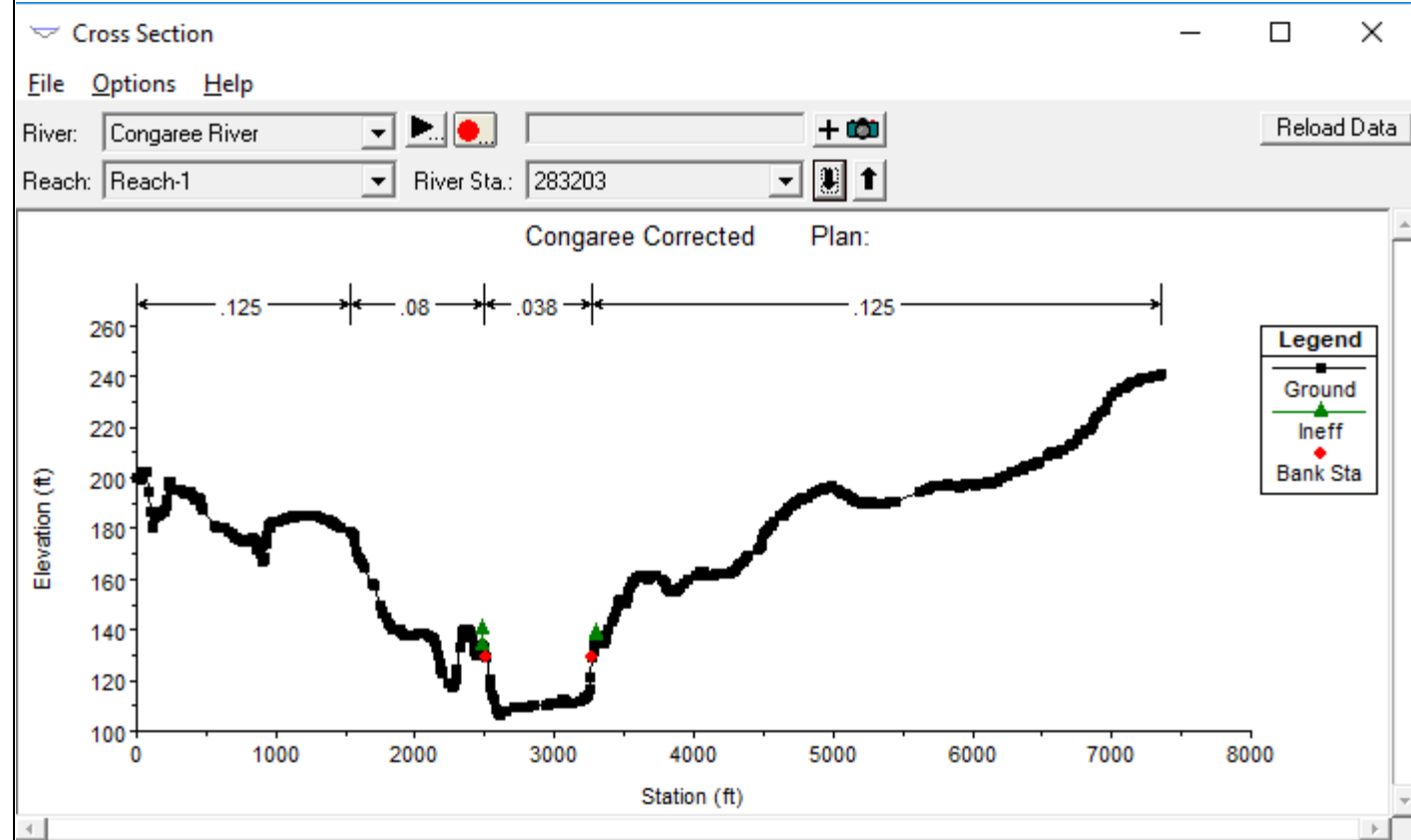
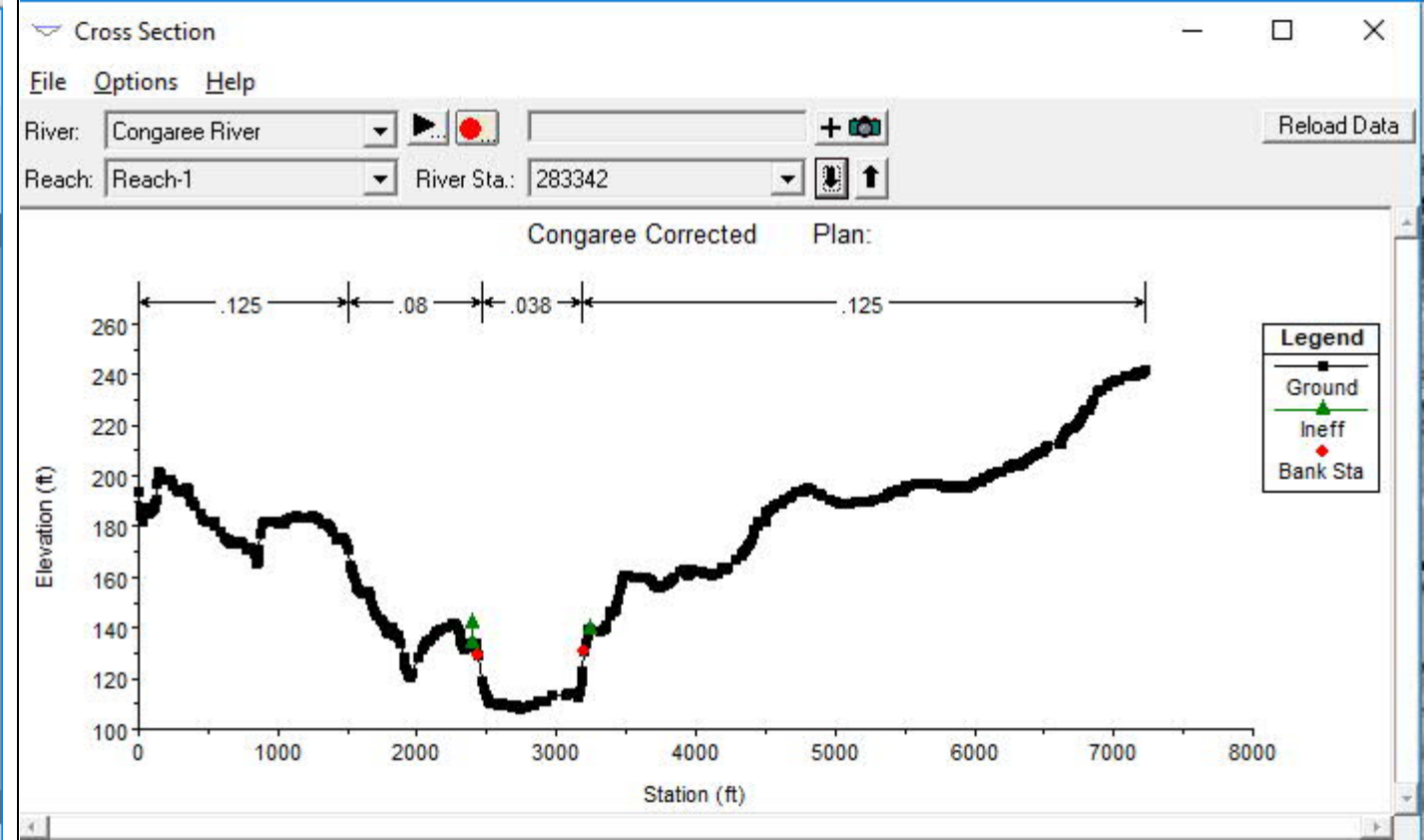
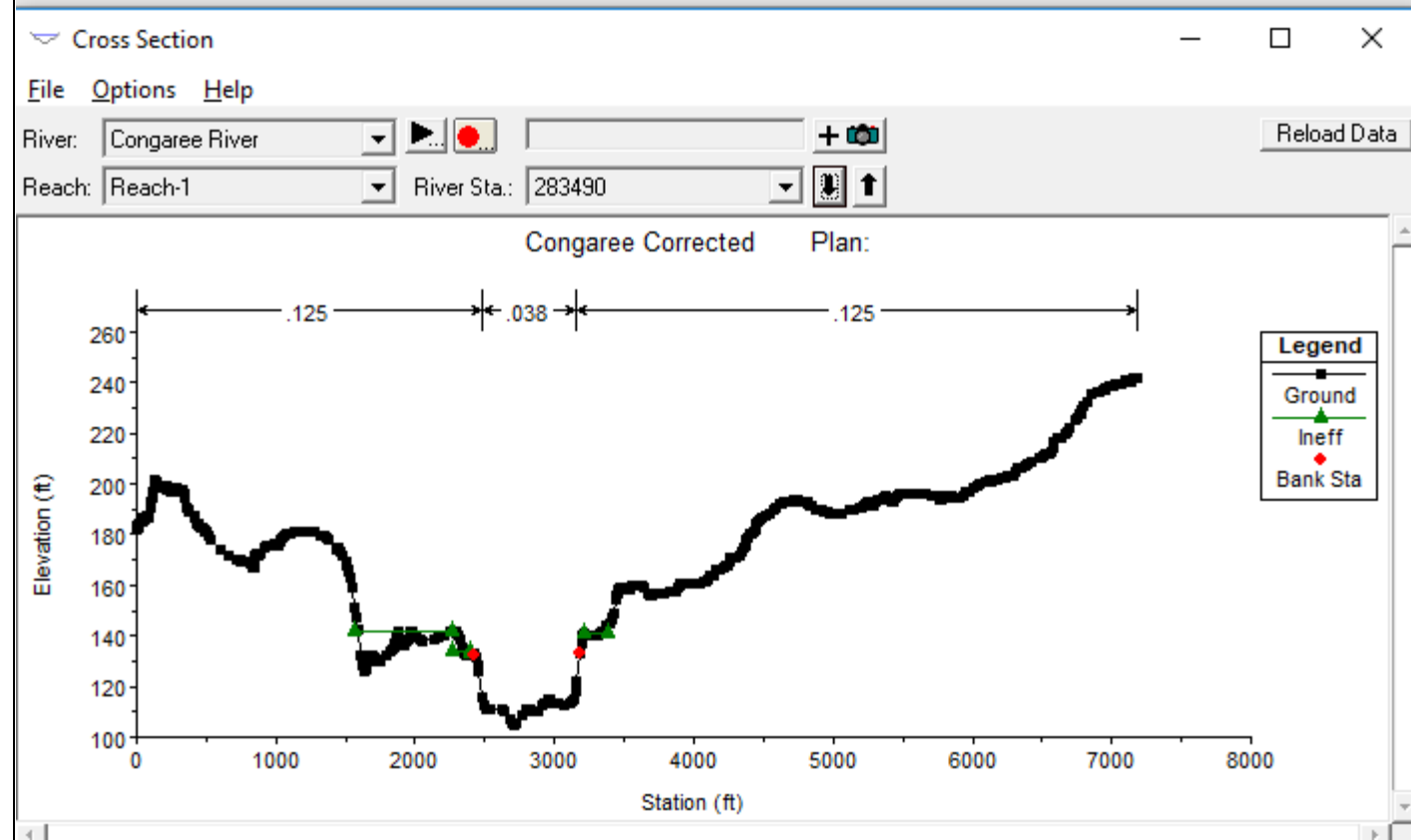


Appendix C: Corrected Effective Model Cross Sections (Sheet 3 of 8)

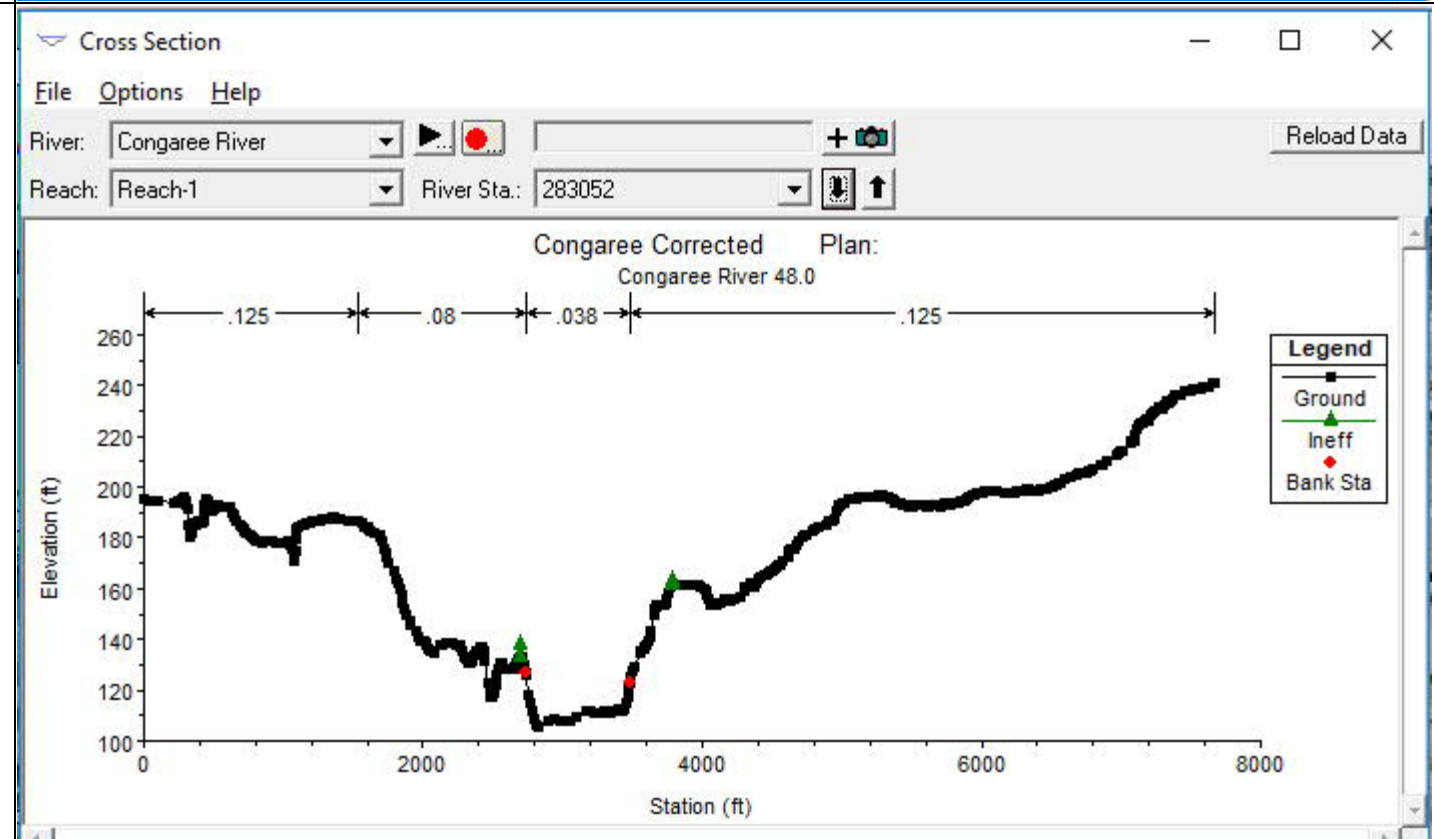
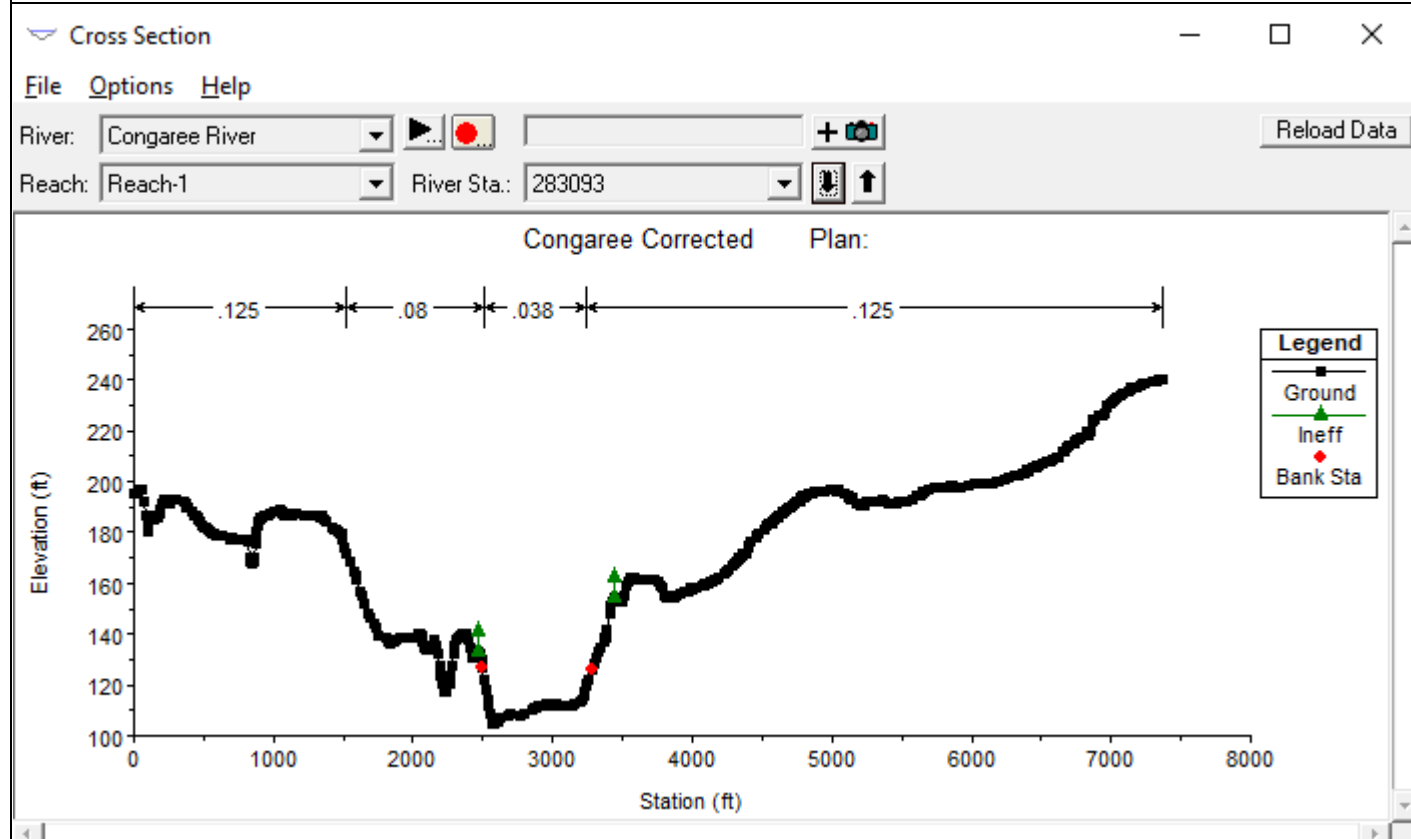
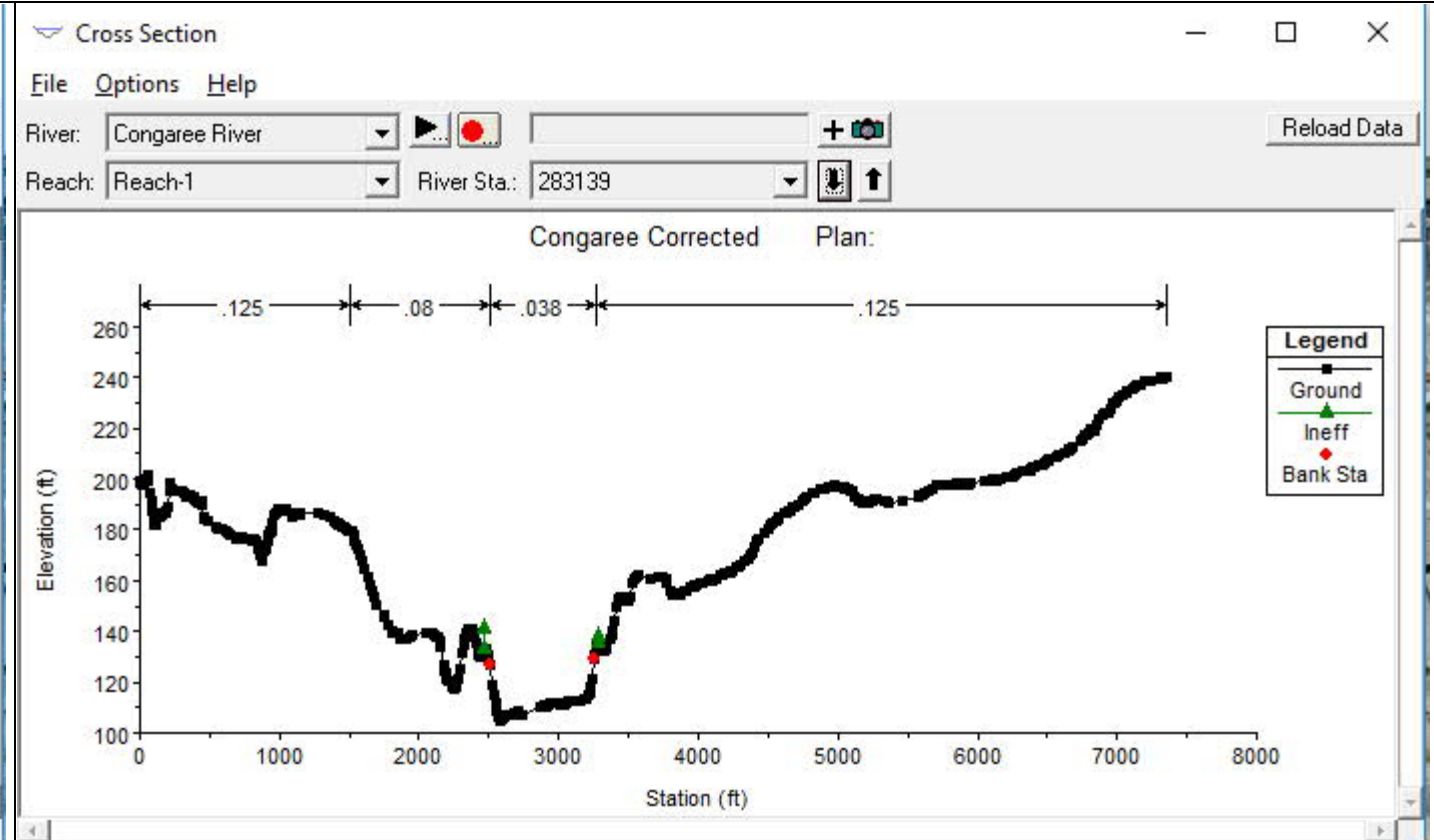
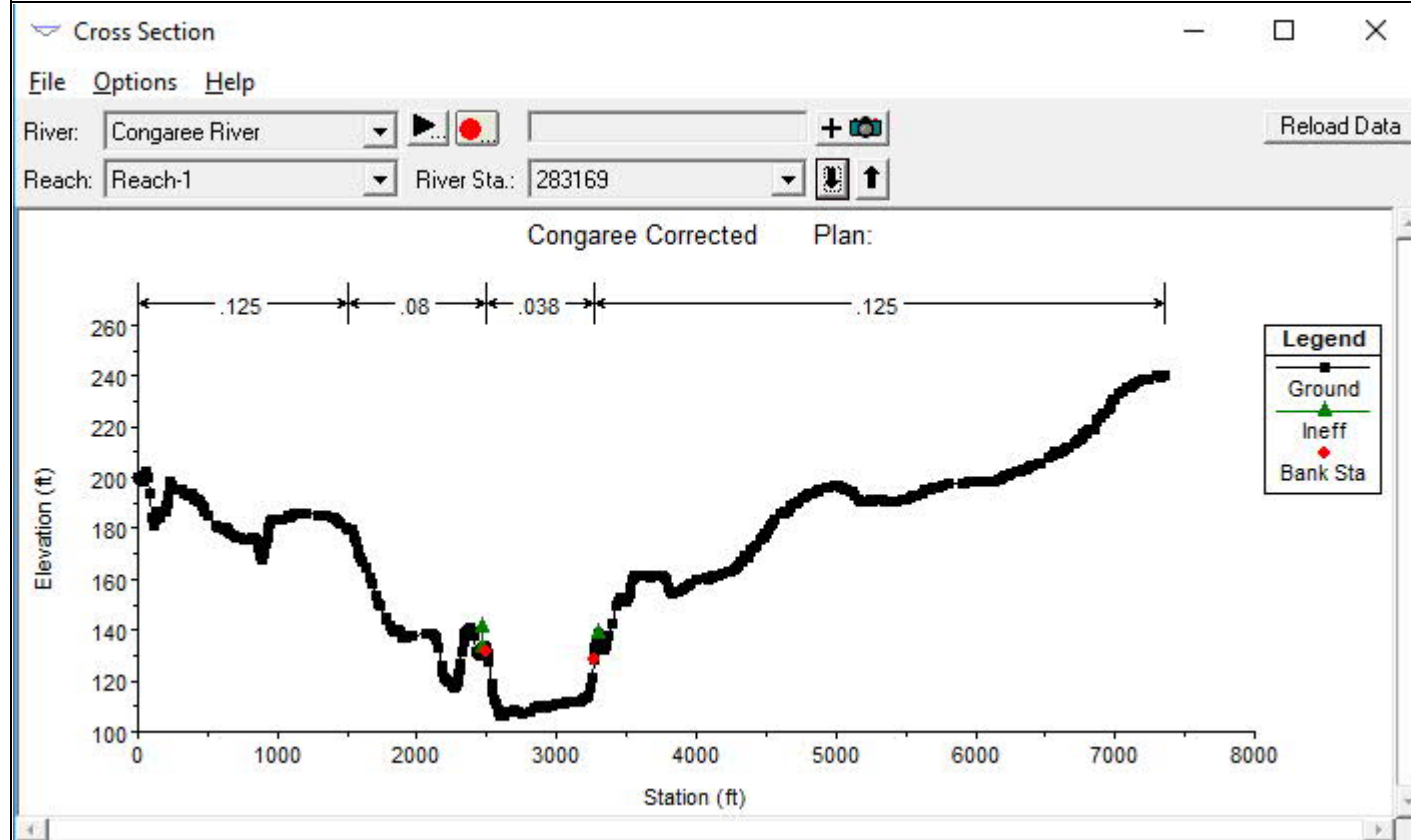




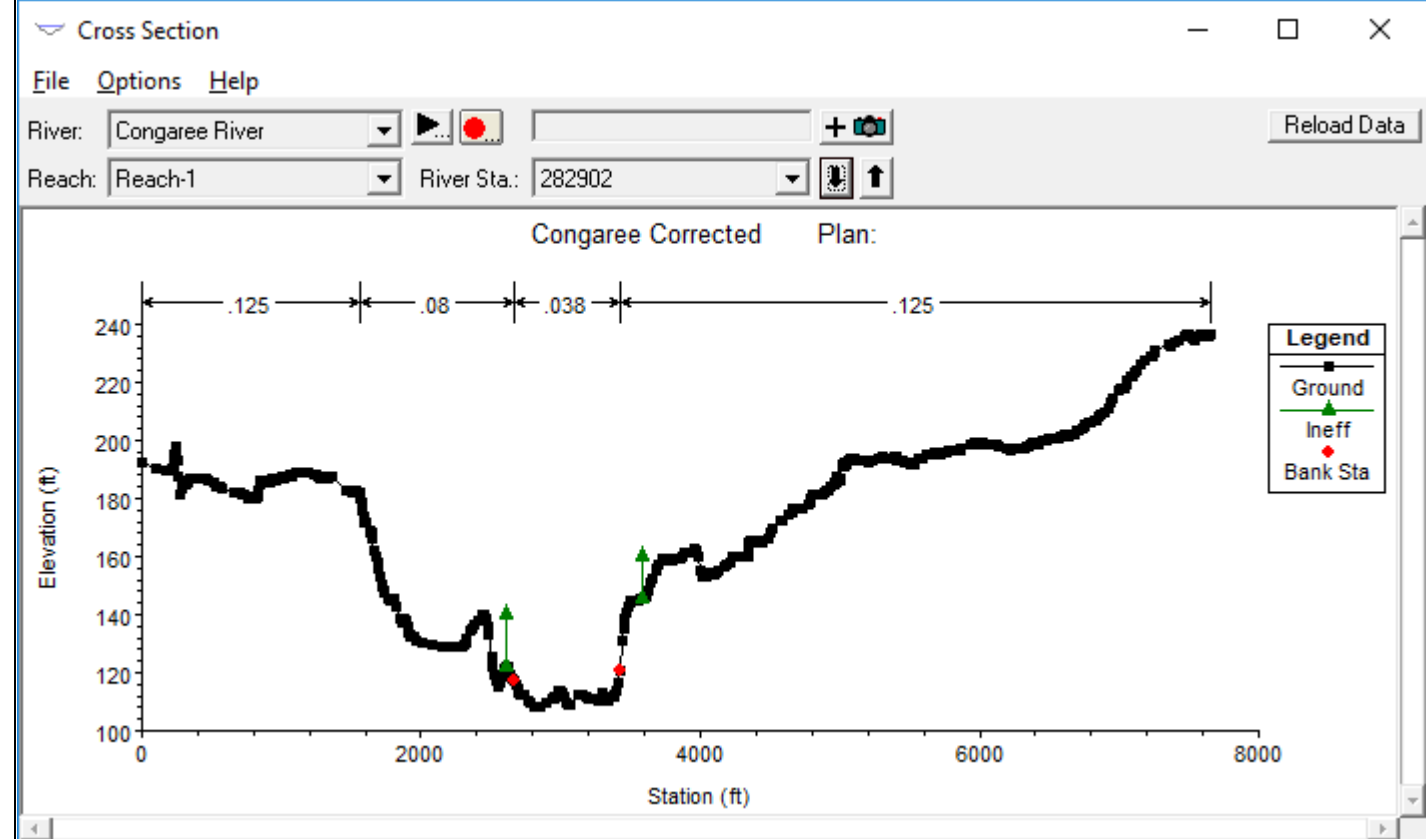
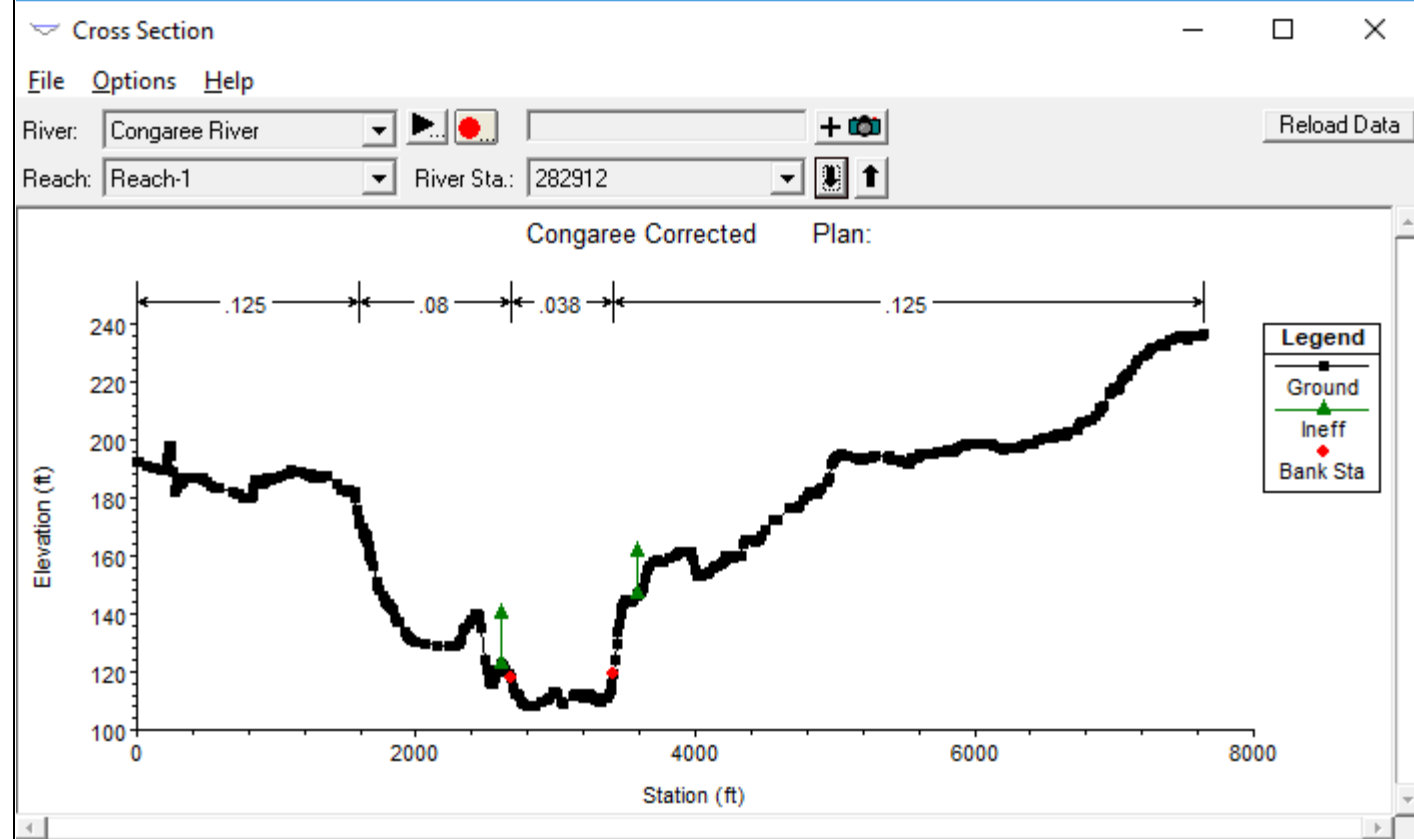
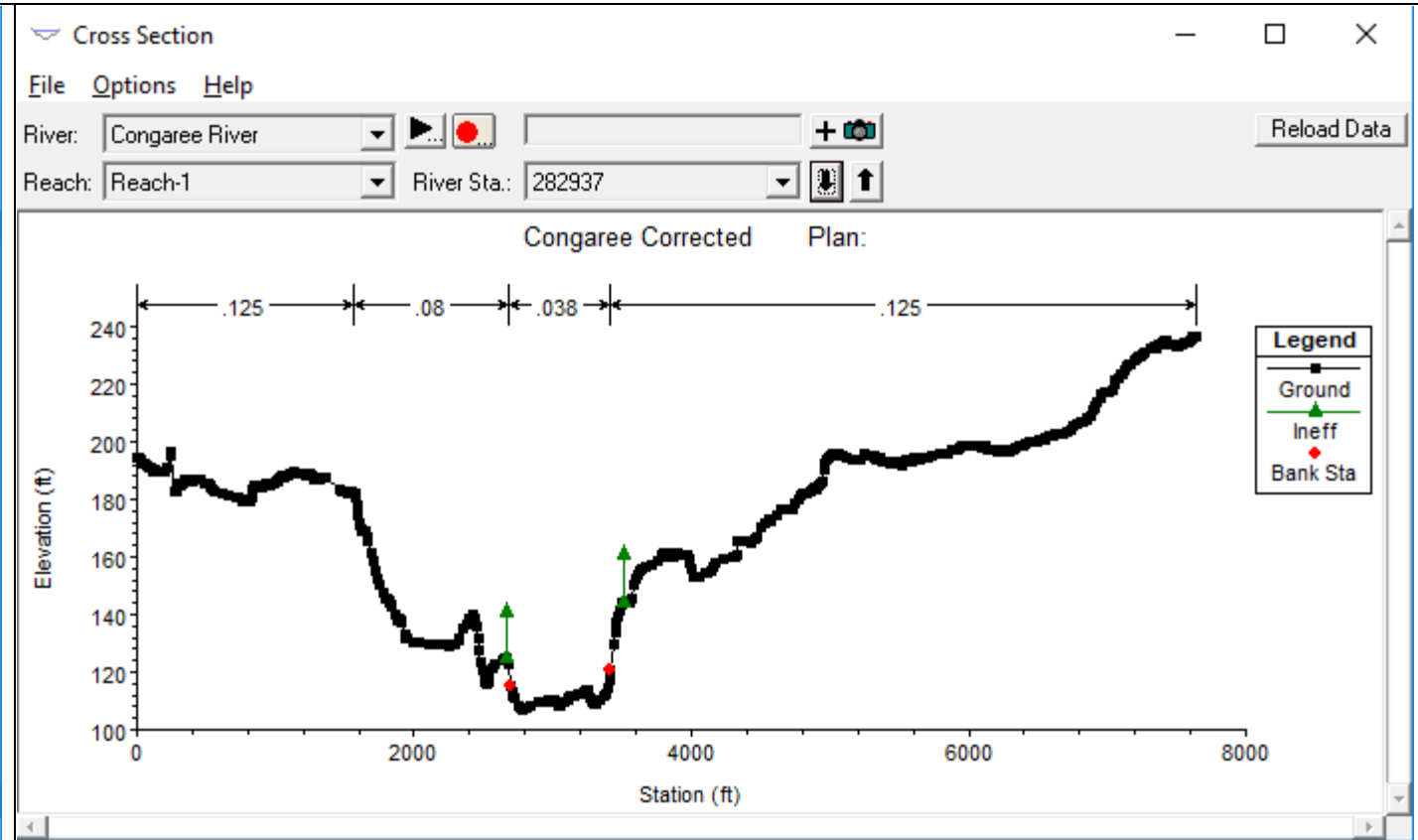
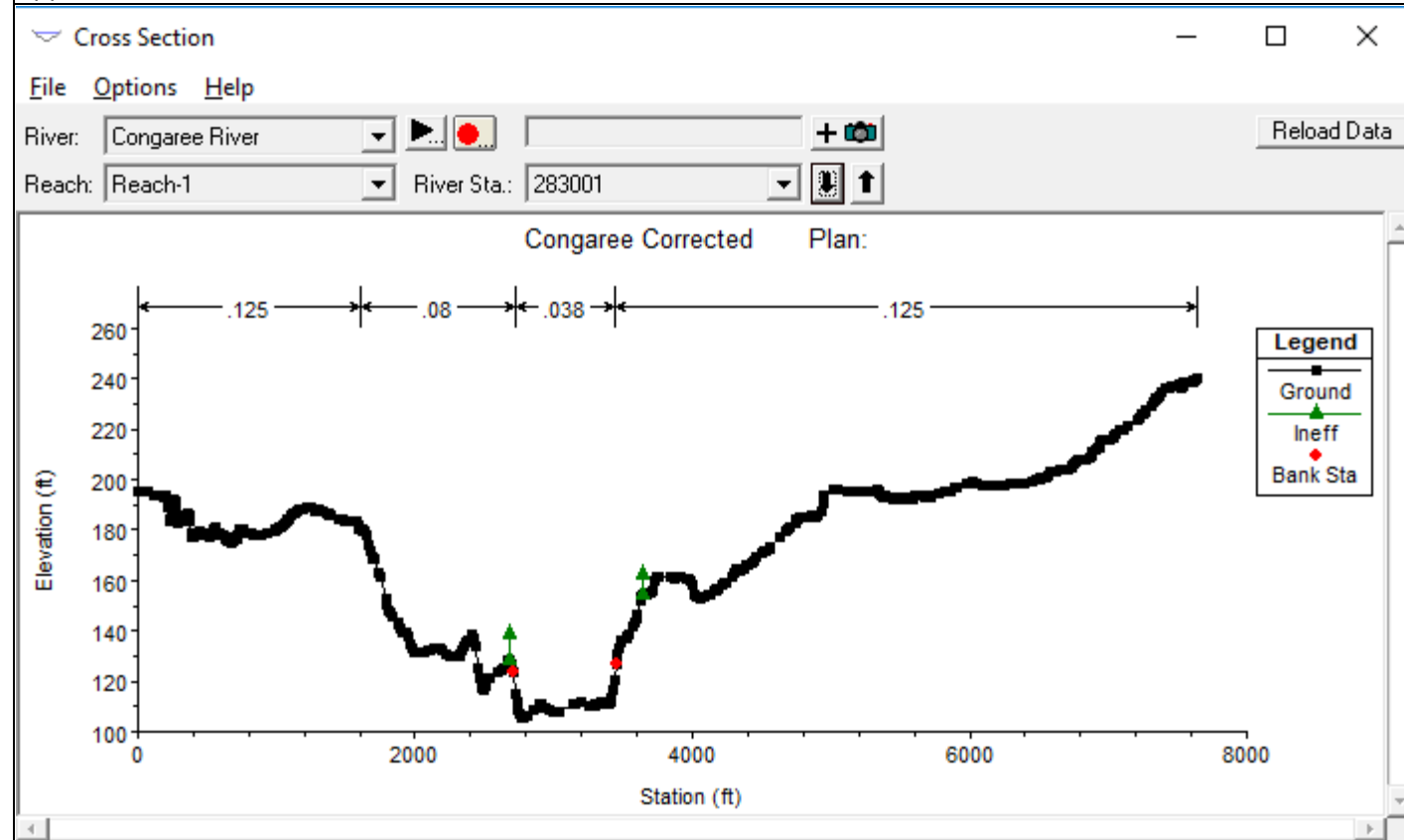
Appendix C: Corrected Effective Model Cross Sections (Sheet 4 of 8)



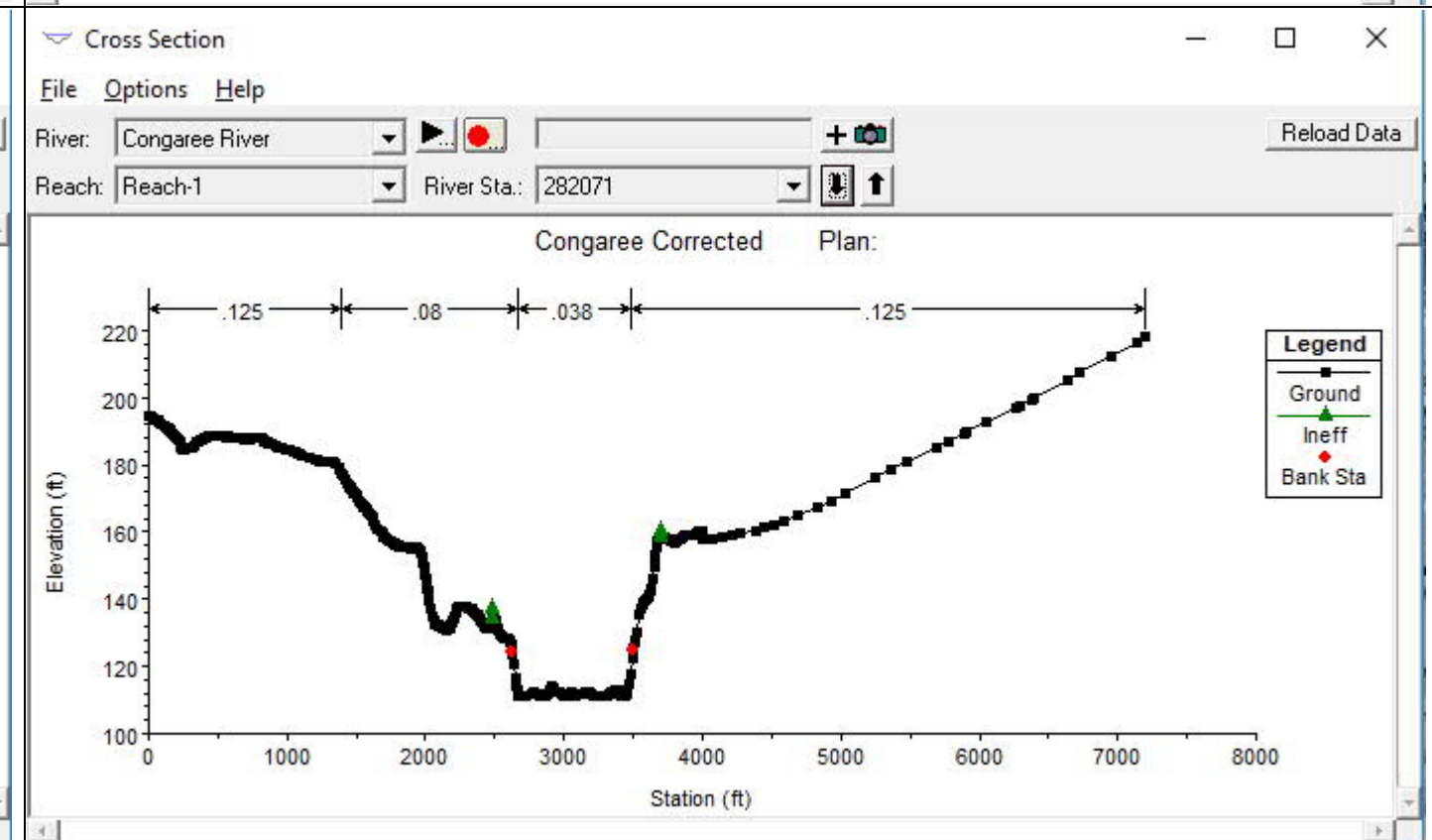
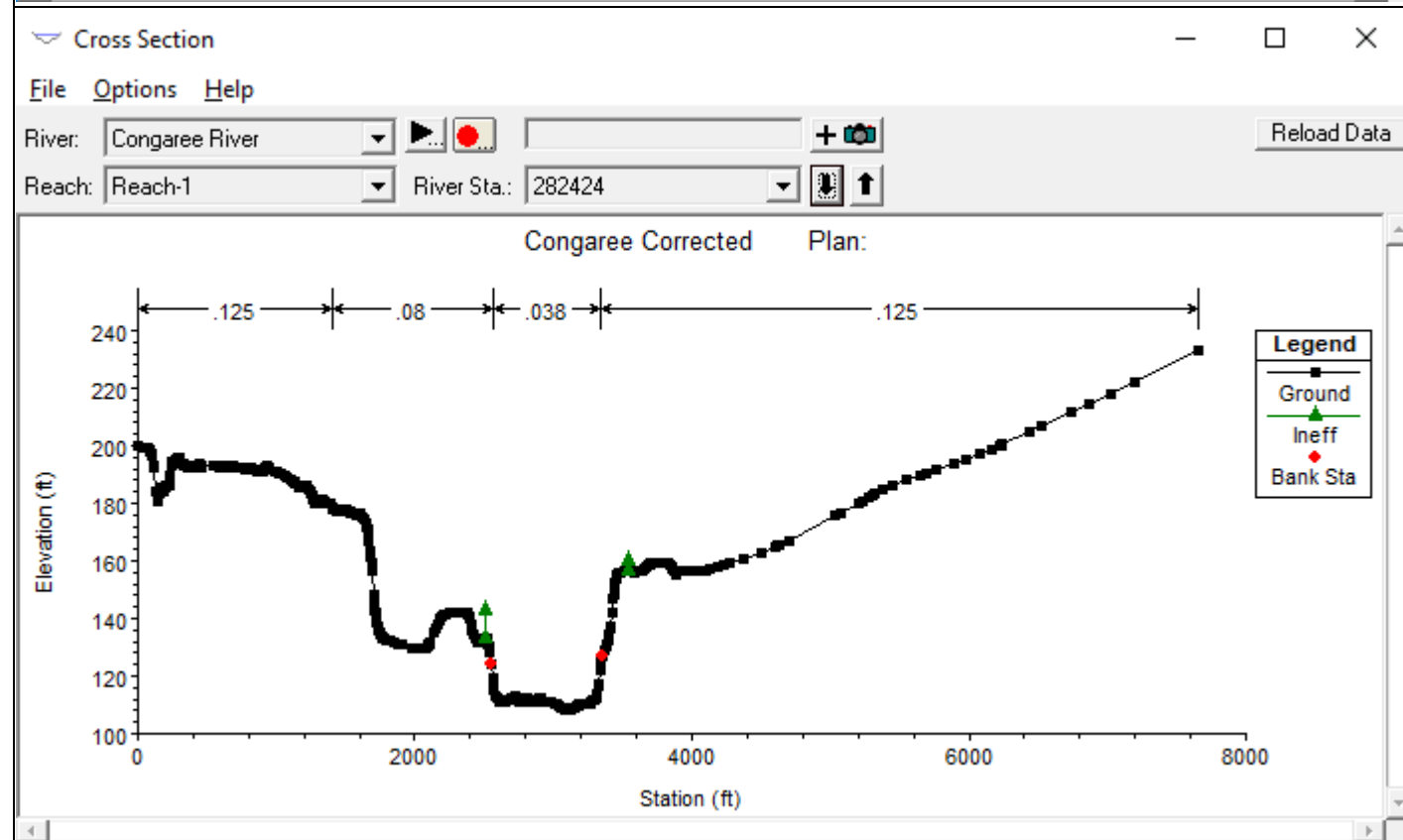
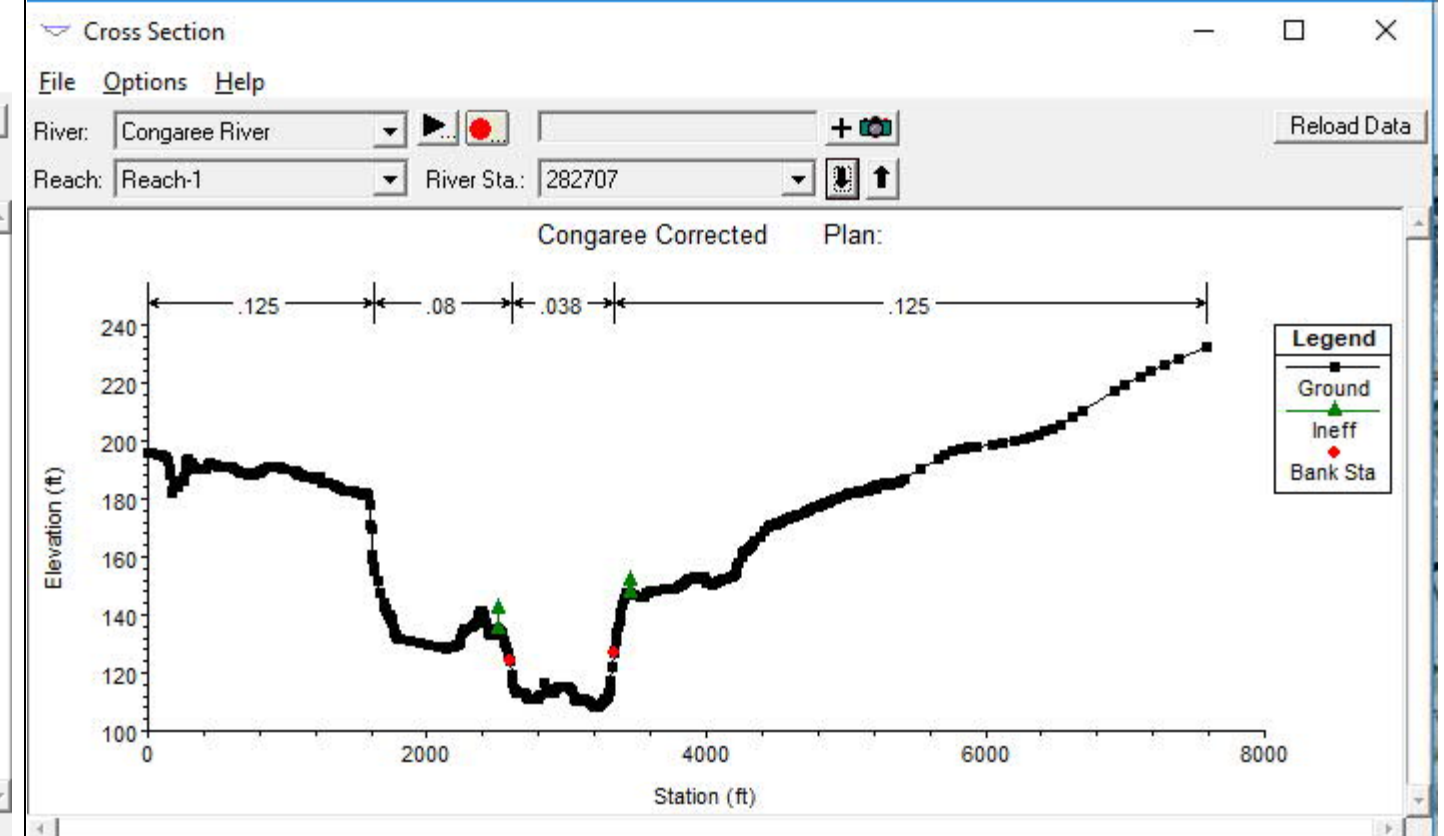
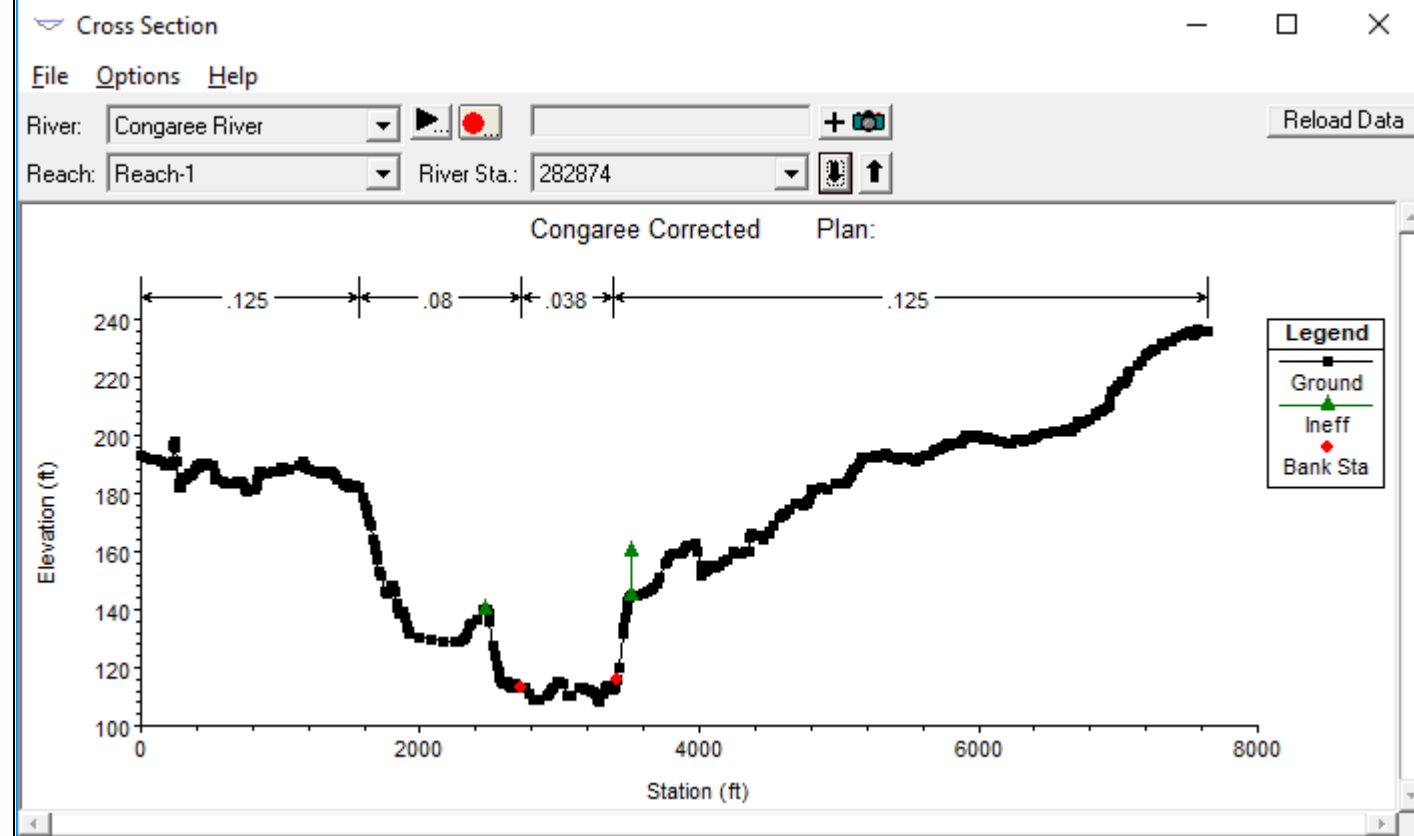
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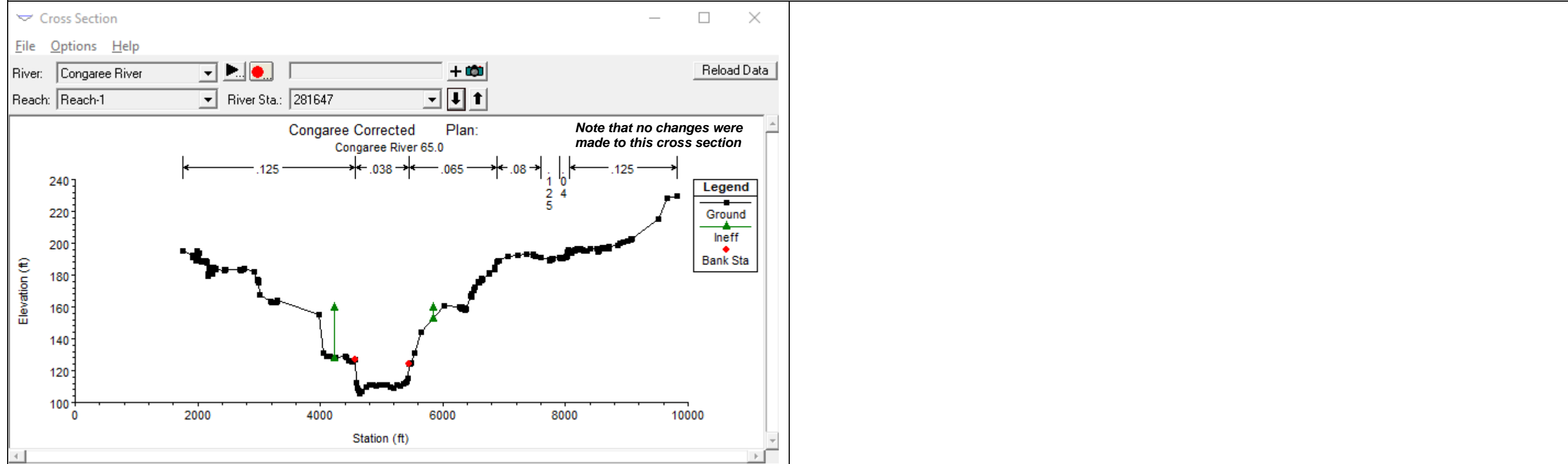
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Appendix C: Corrected Effective Model Cross Sections (Sheet 7 of 8)



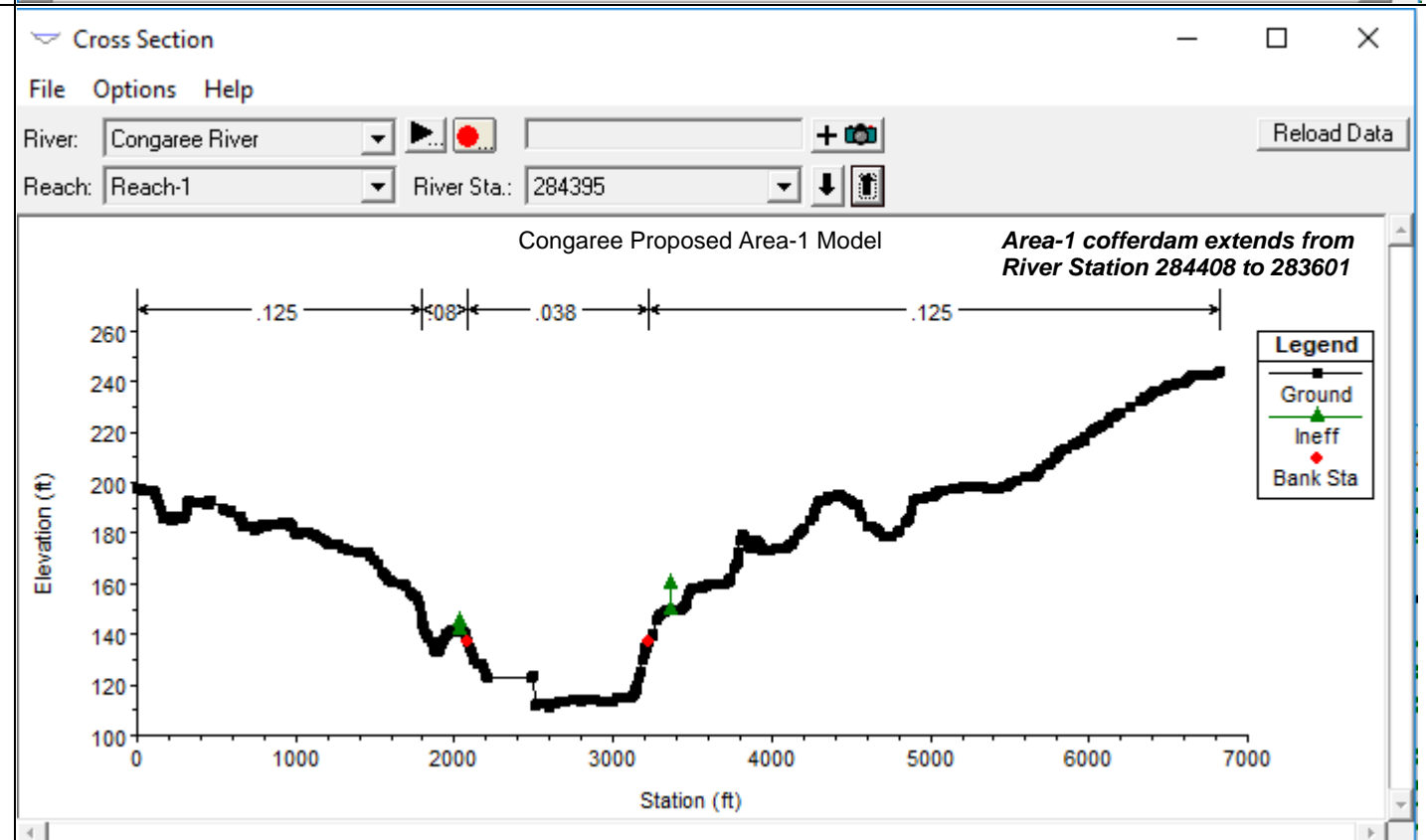
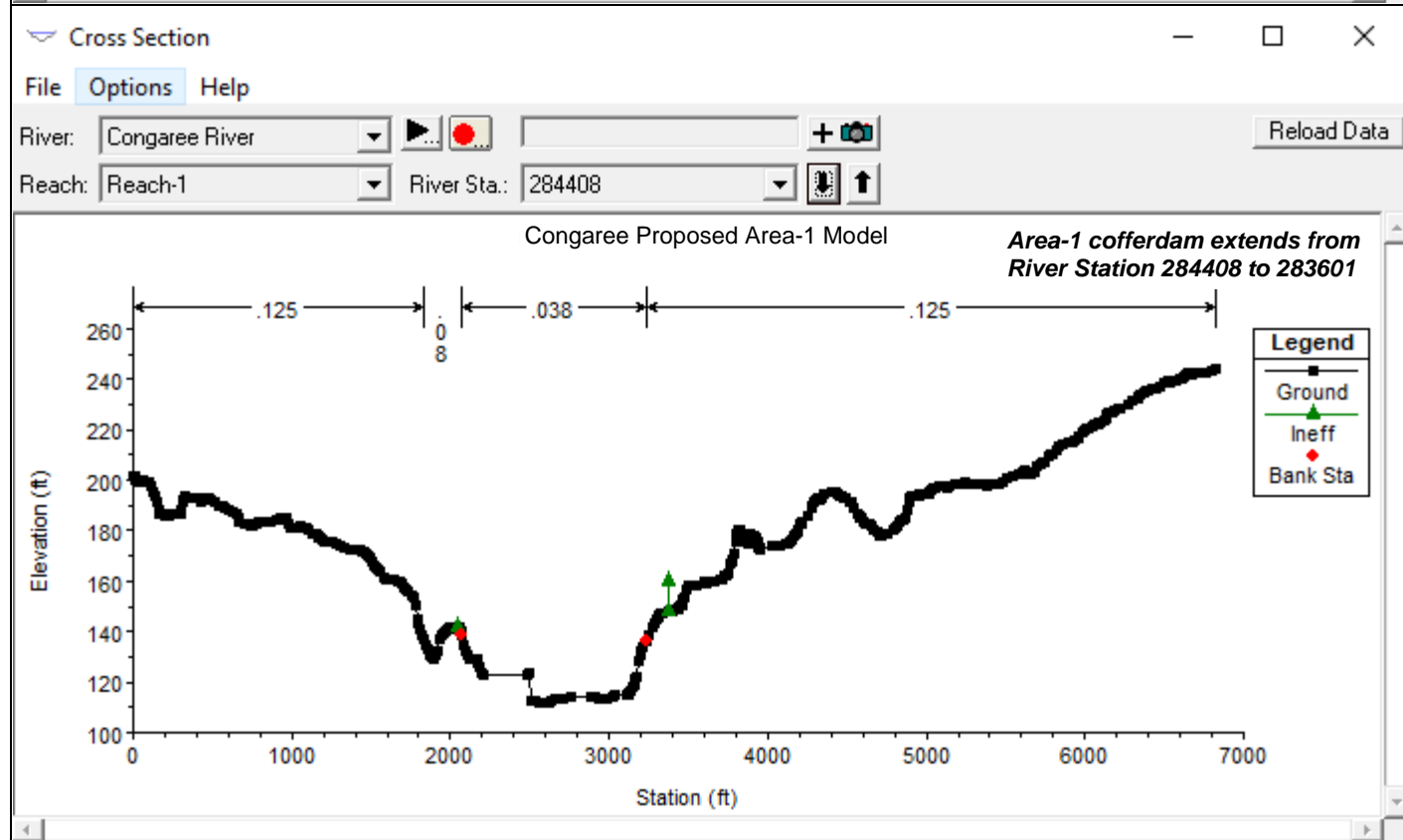
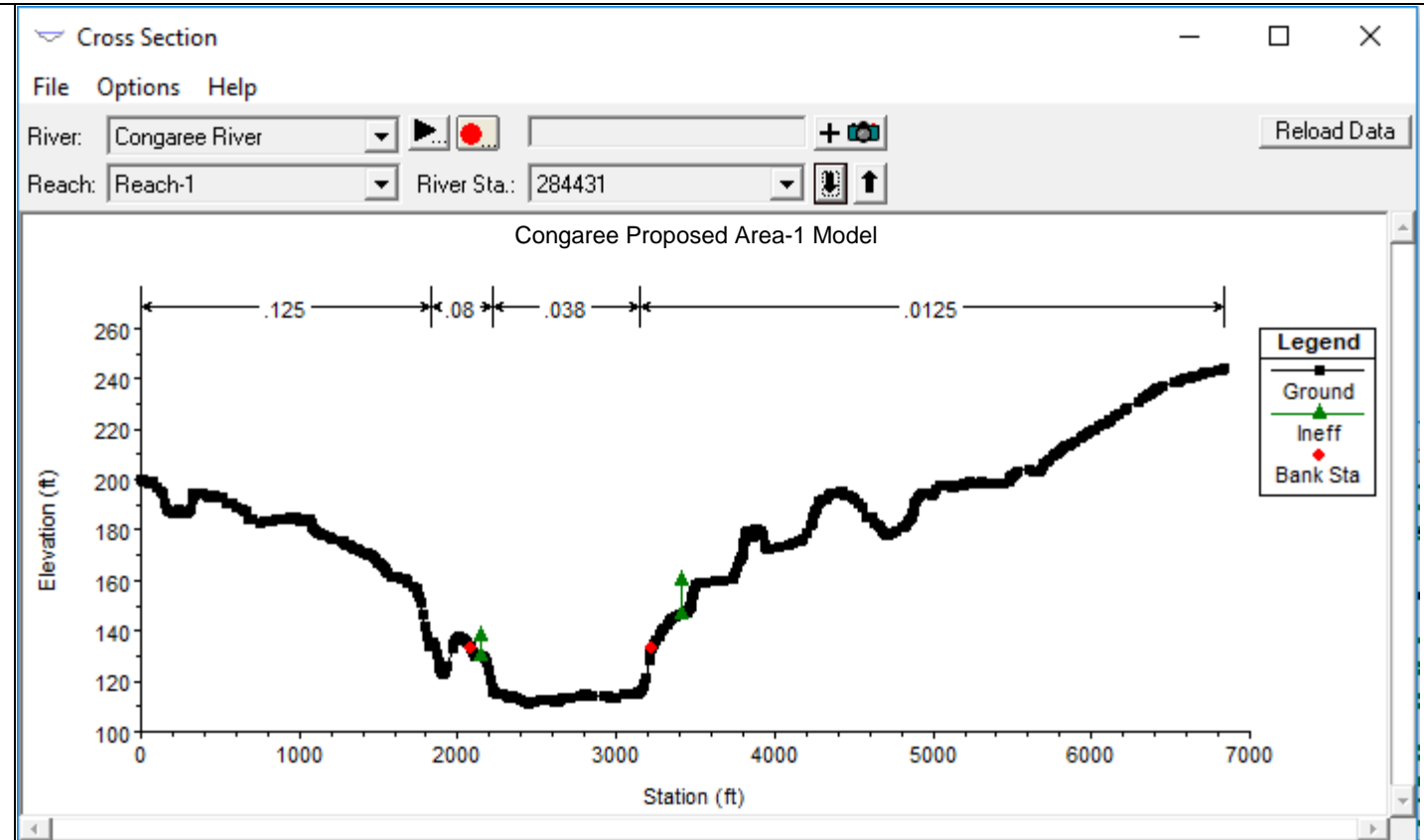
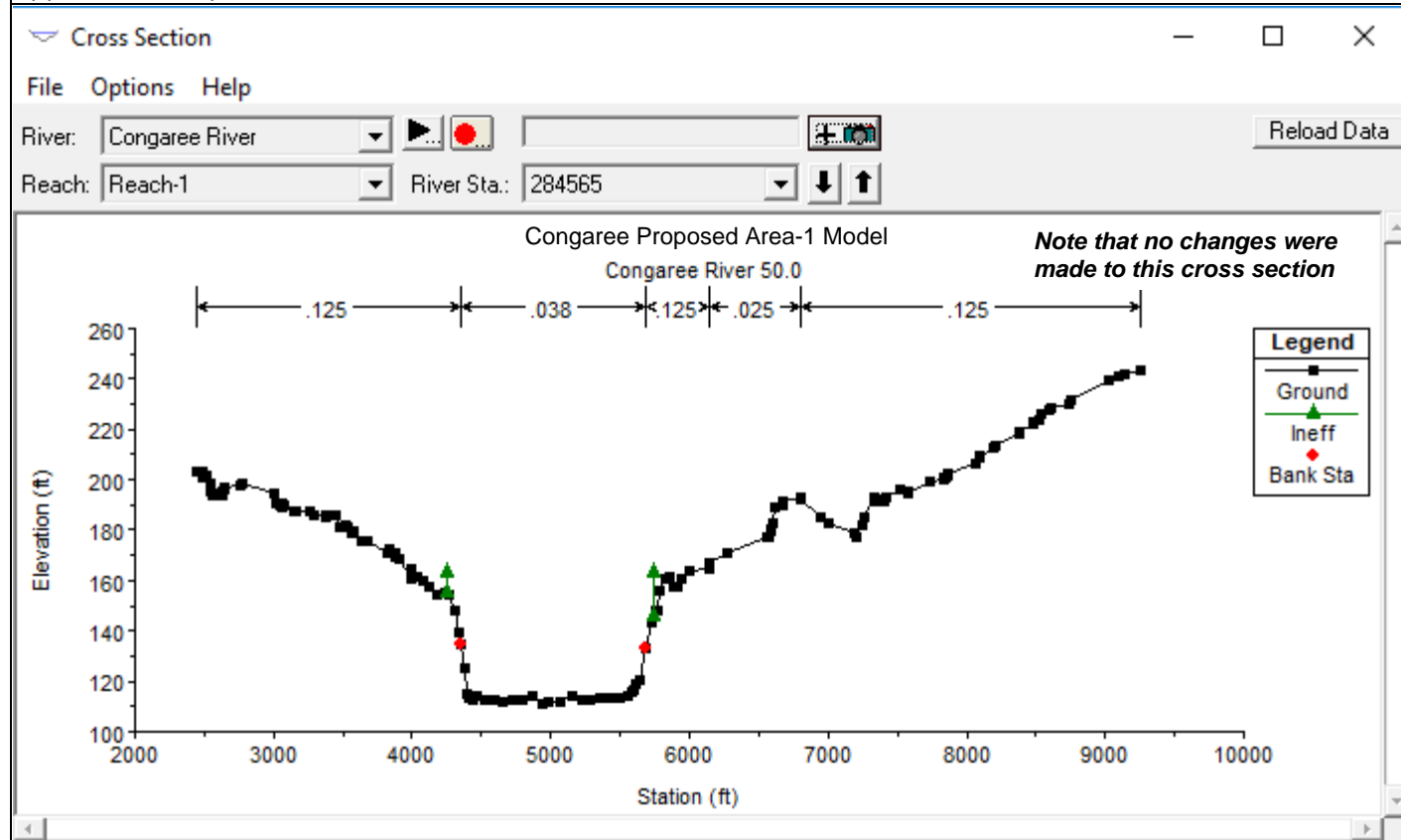
Appendix C: Corrected Effective Model Cross Sections (Sheet 8 of 8)



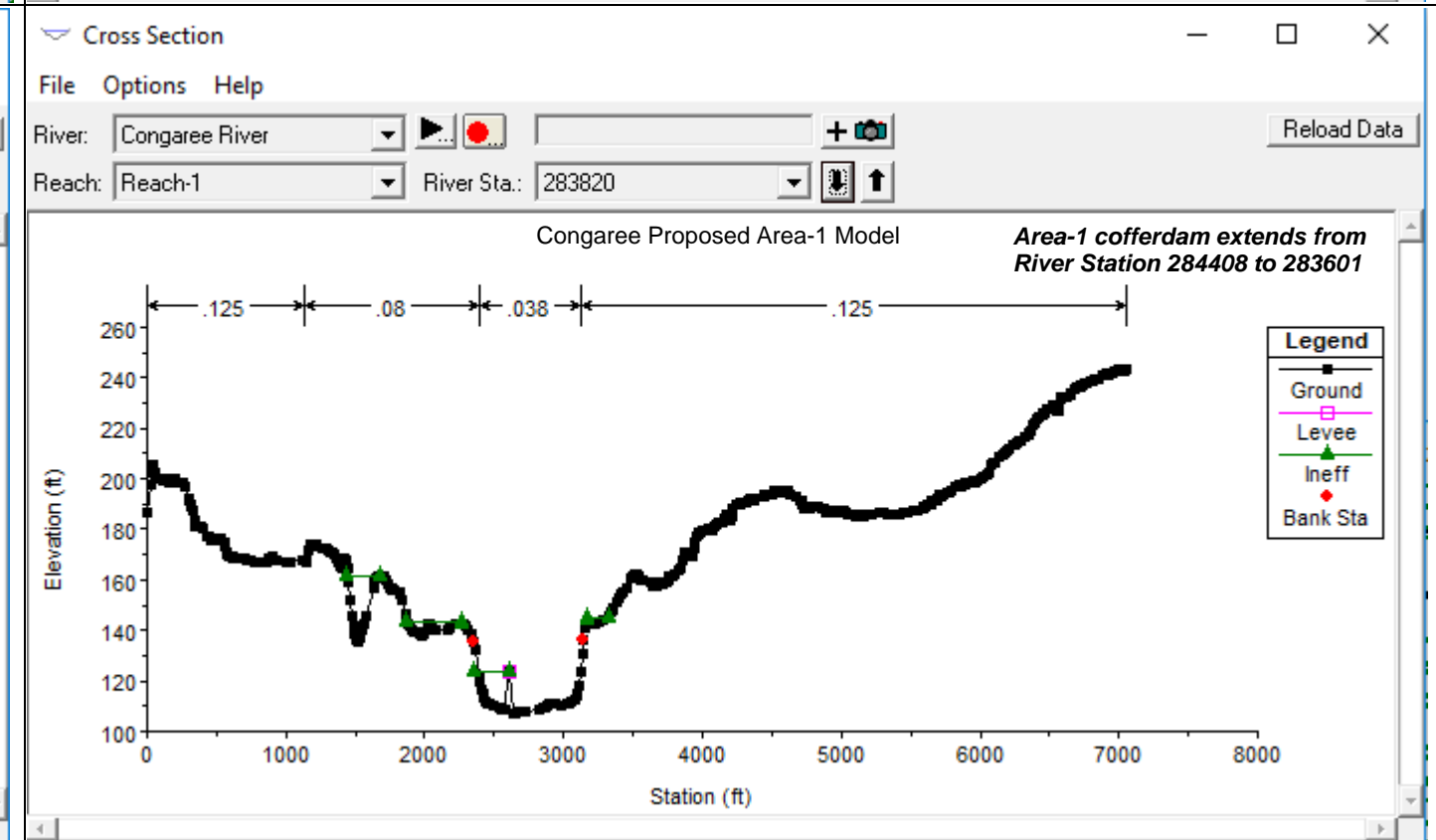
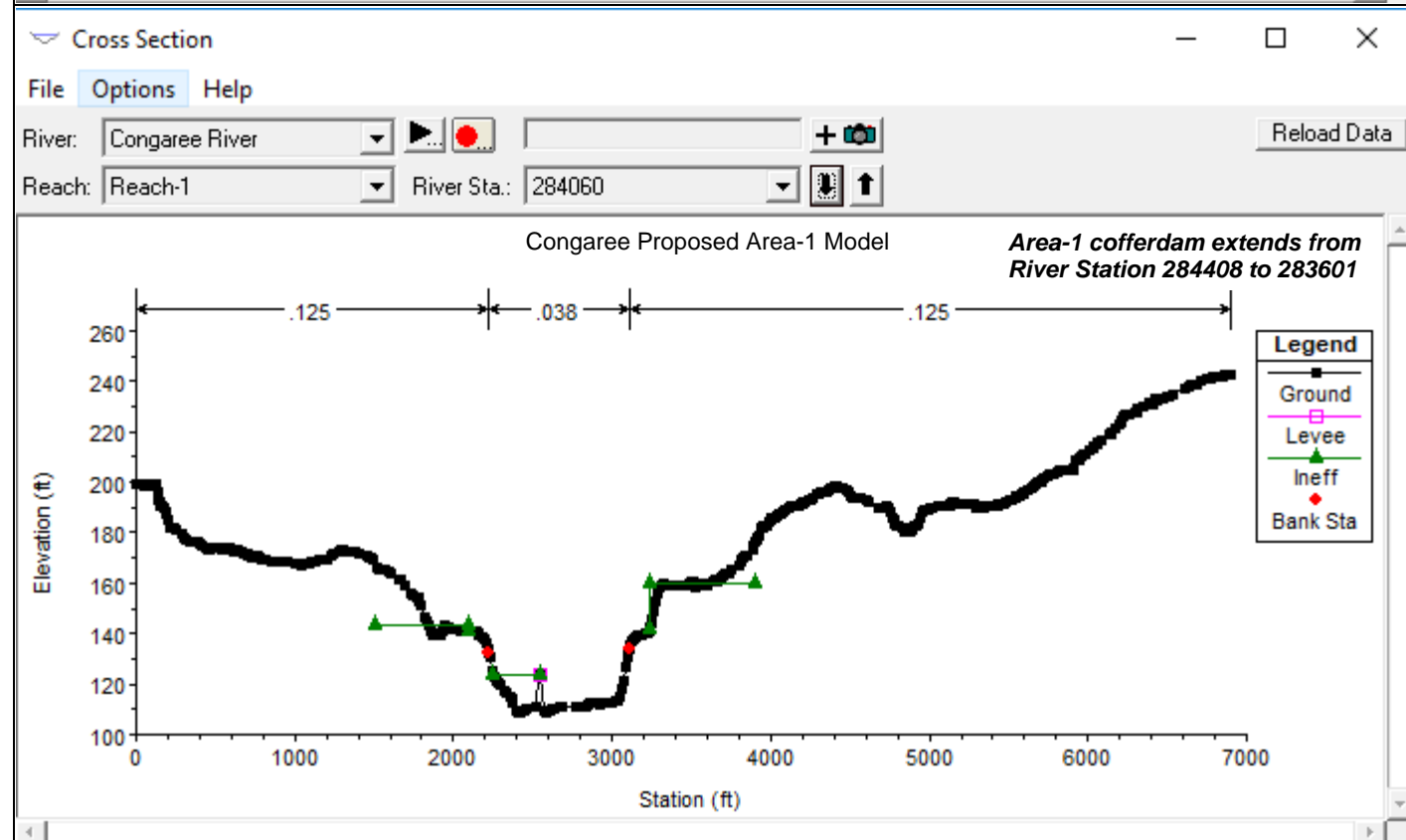
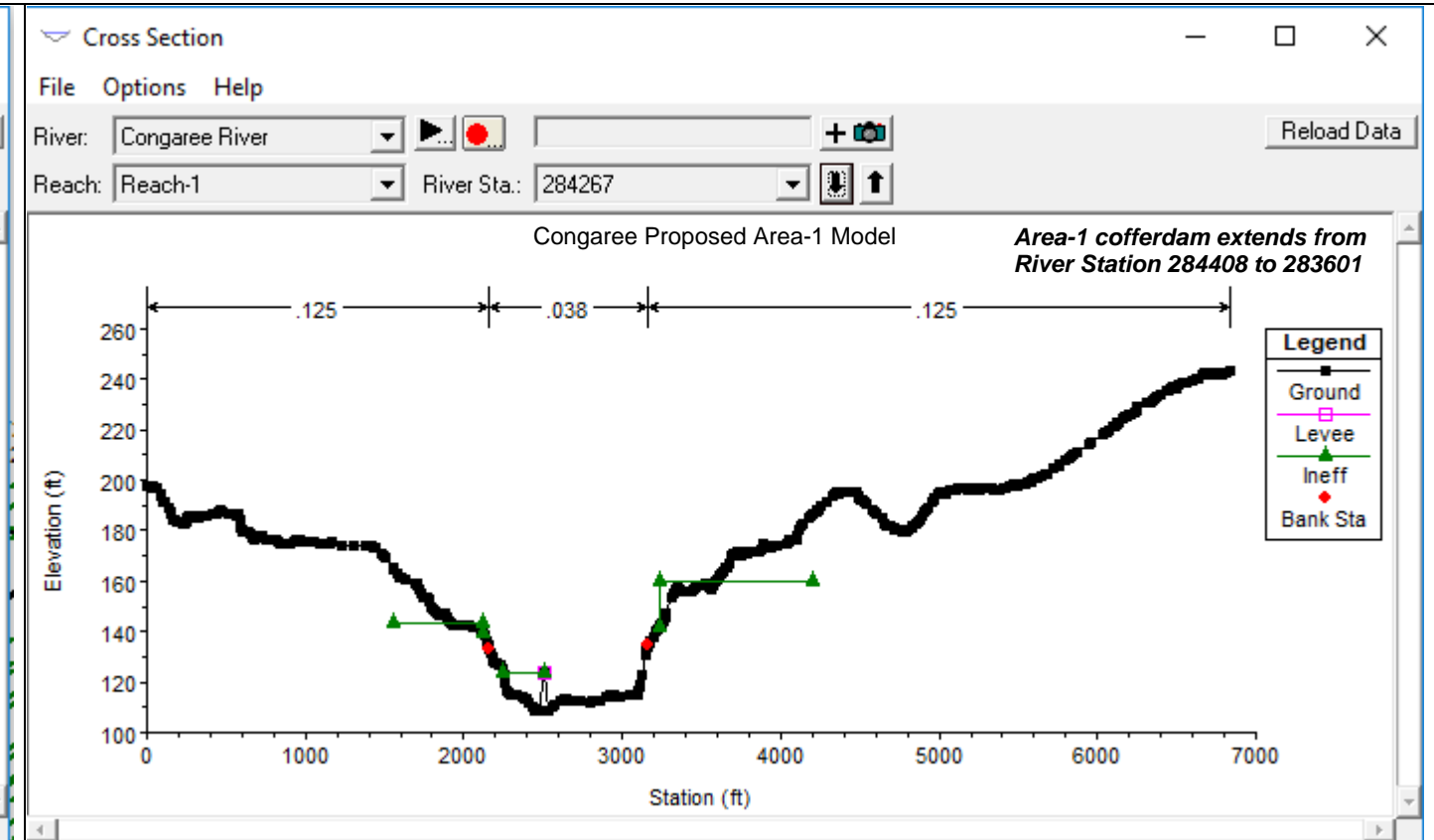
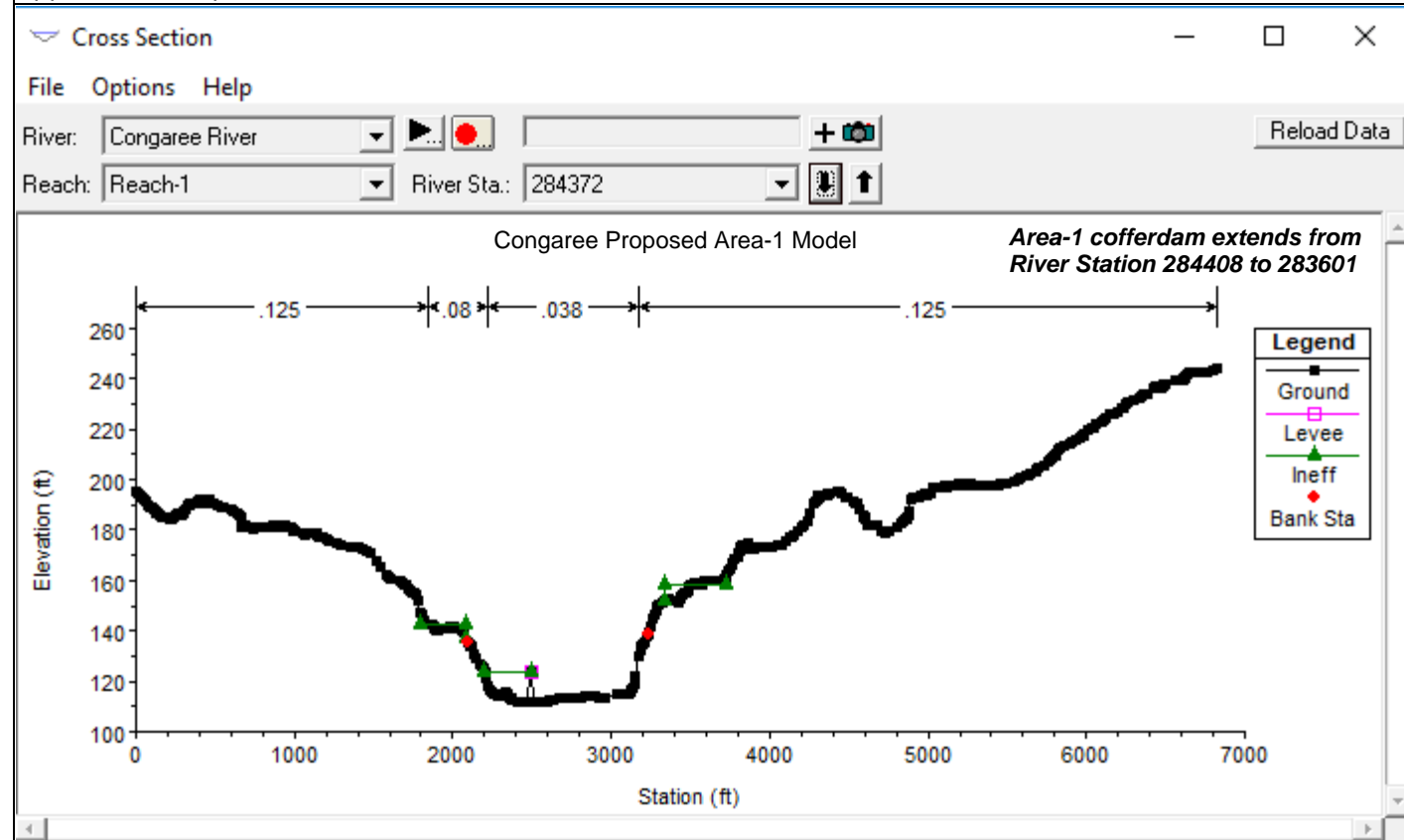


## Appendix D: Proposed Condition Models Cross Sections

Appendix D: Proposed Conditions Models Cross Sections (Sheet 1 of 8)

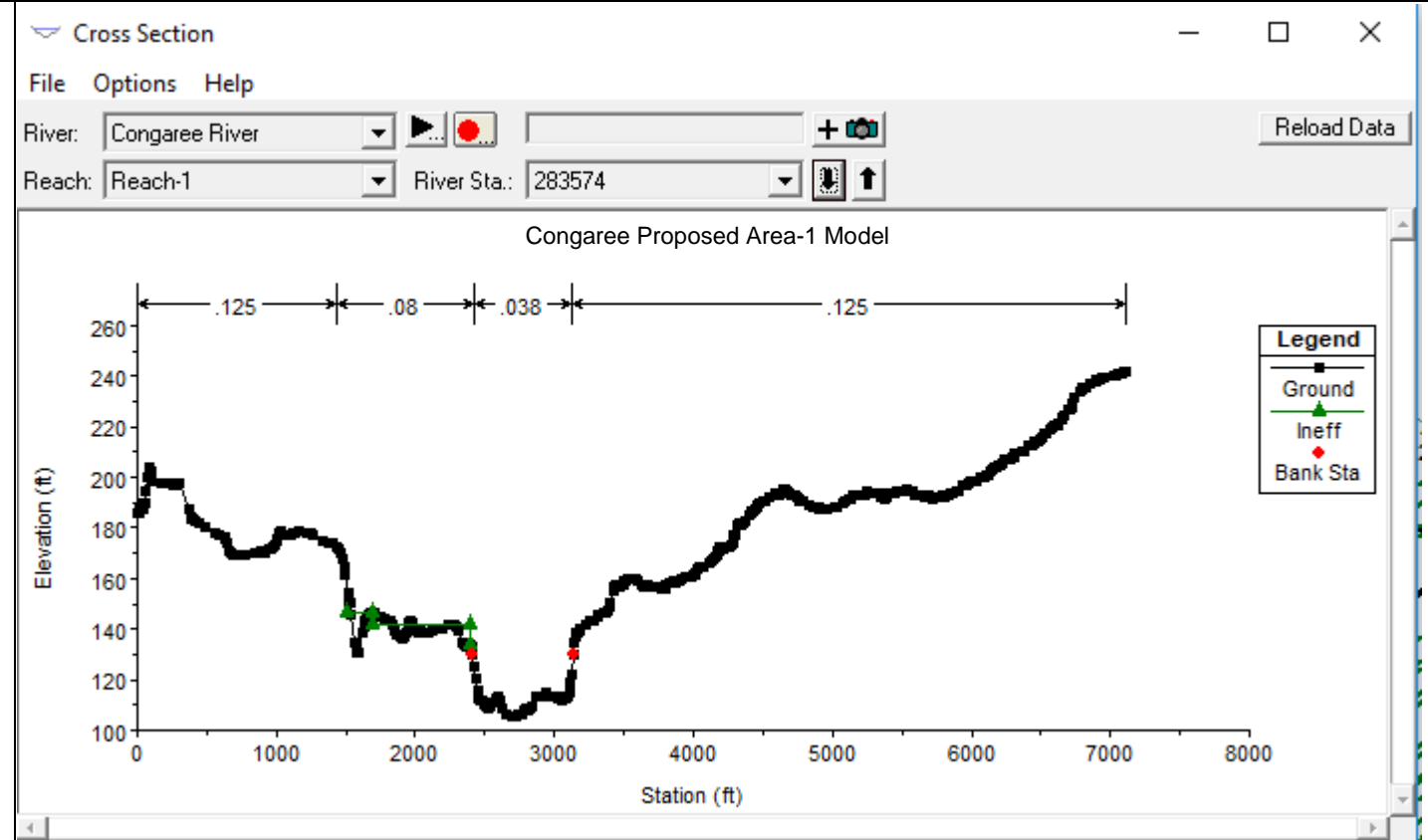
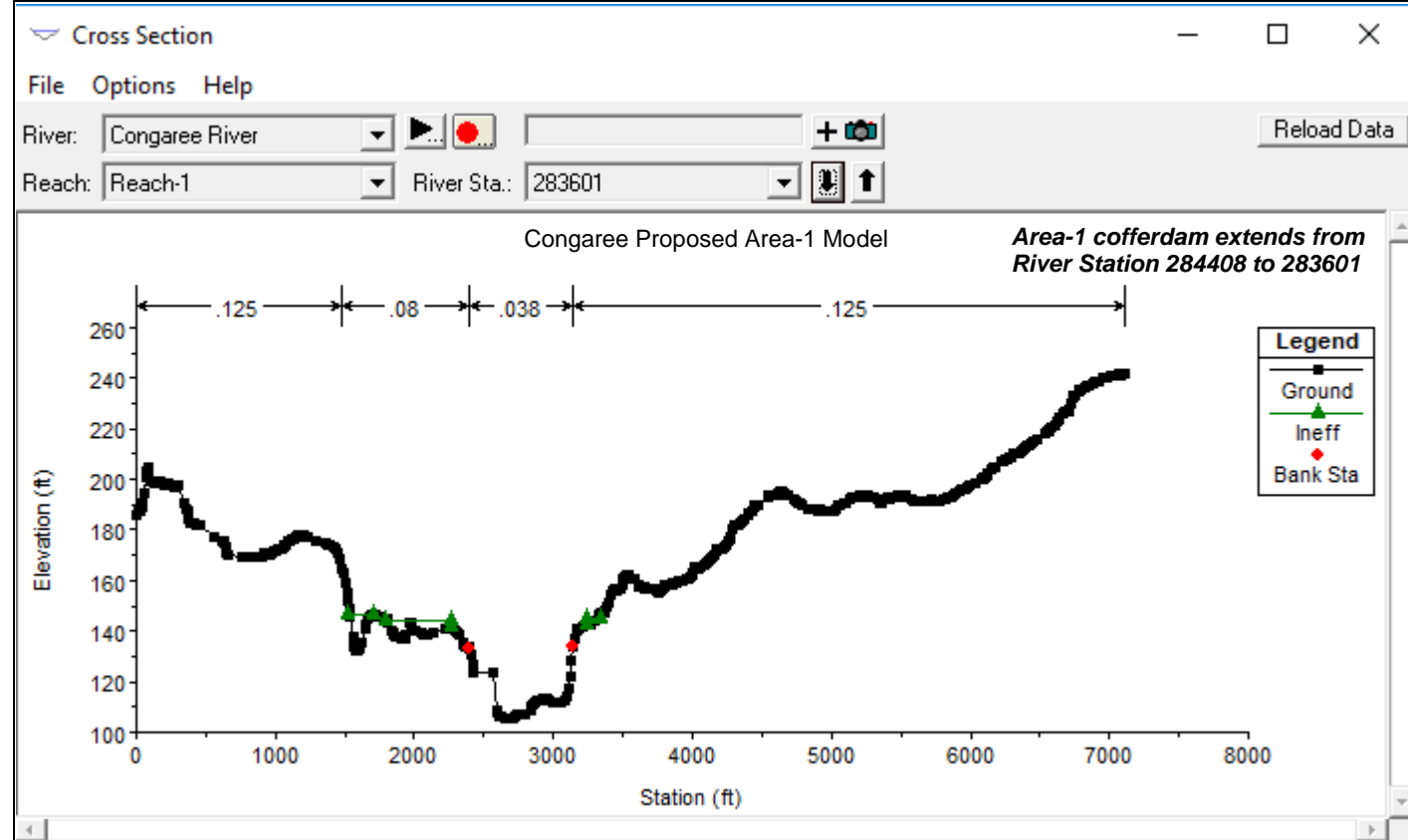
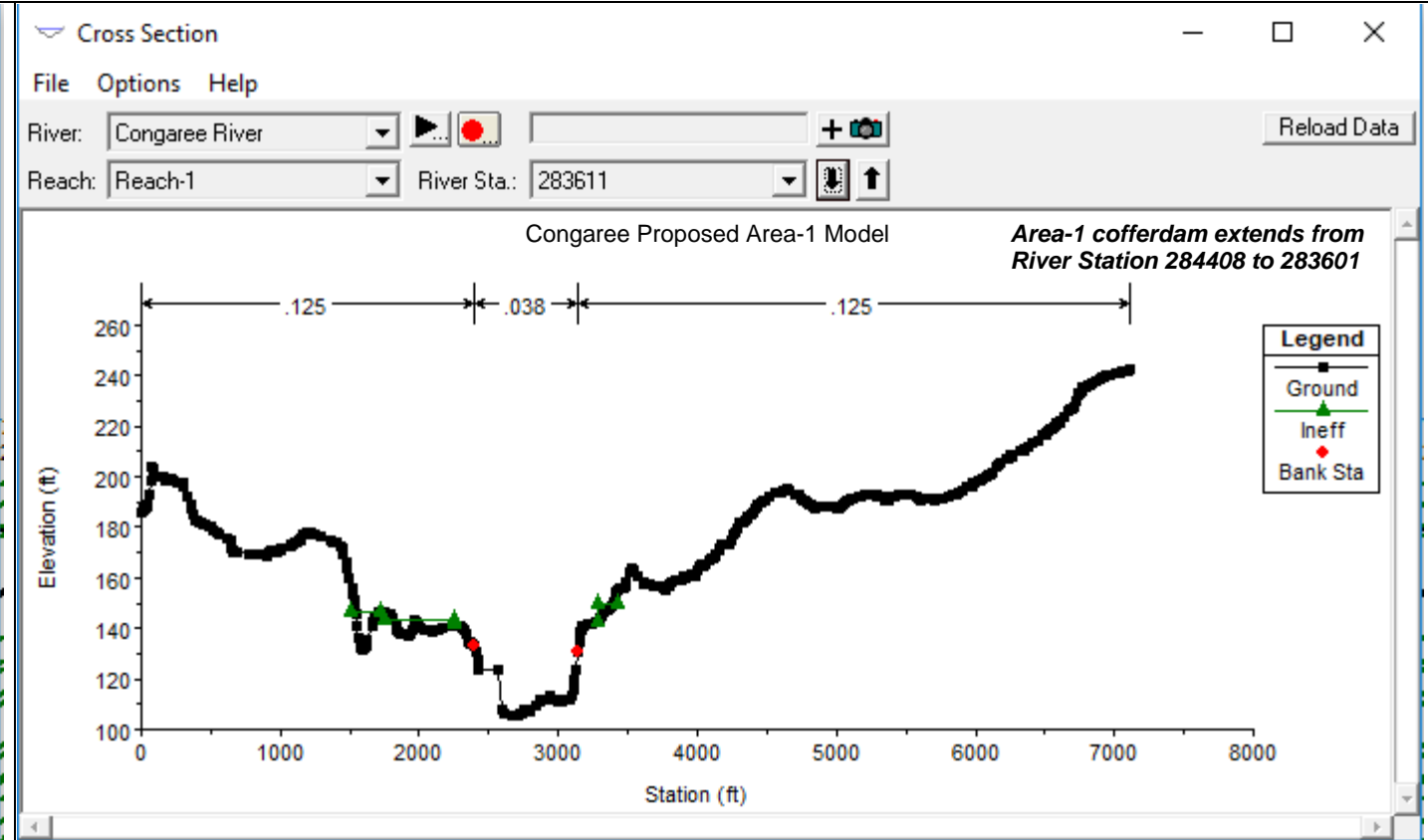
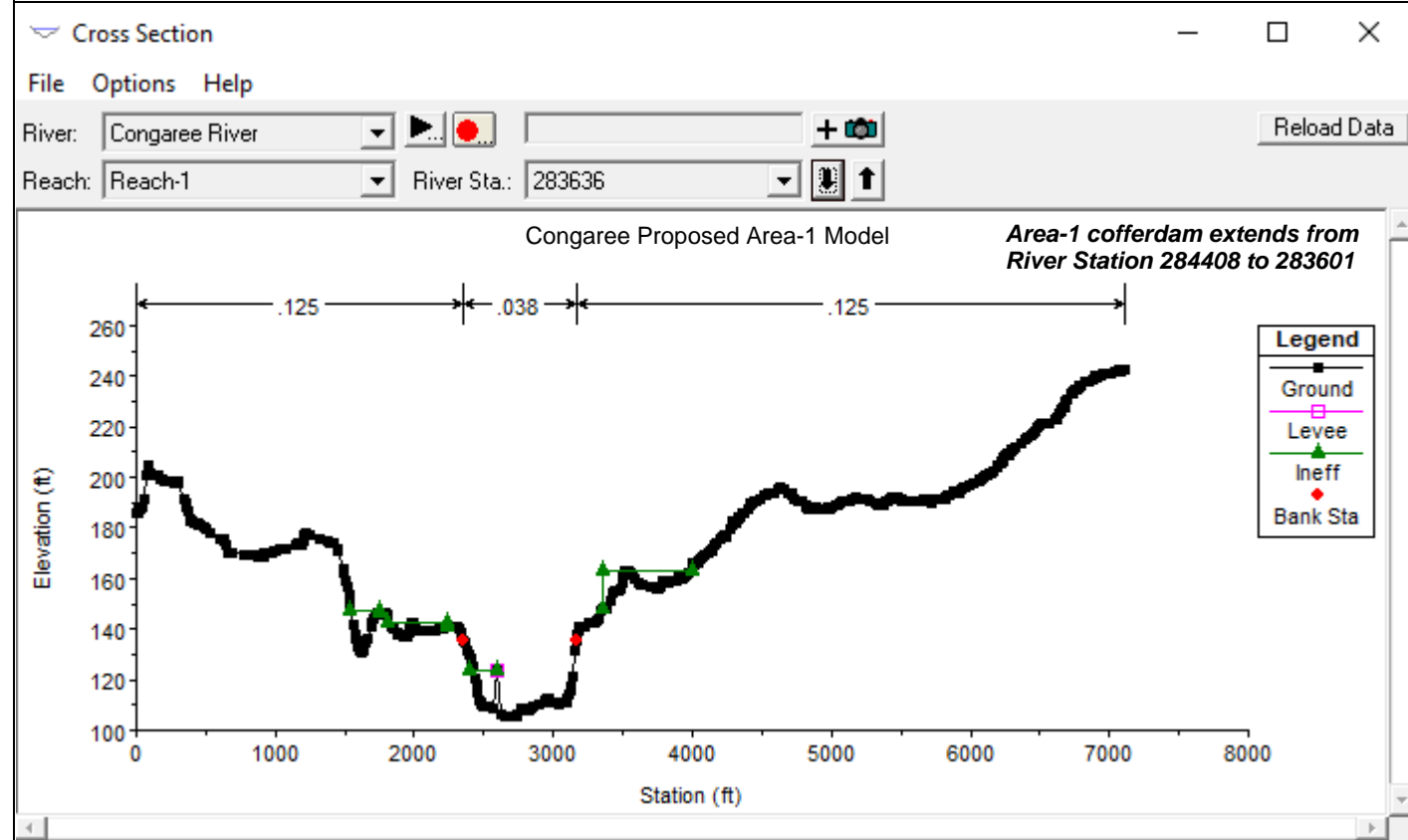


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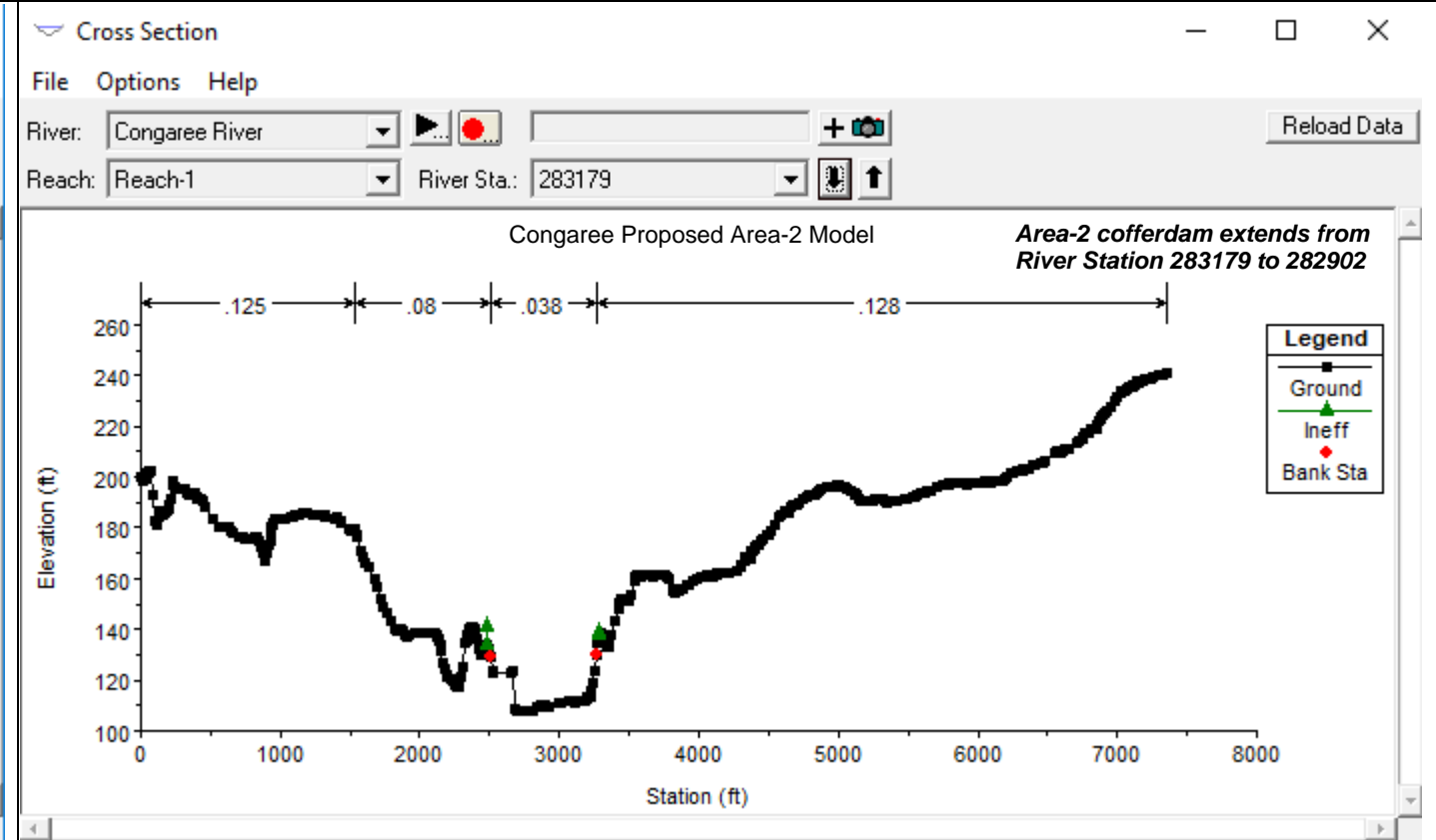
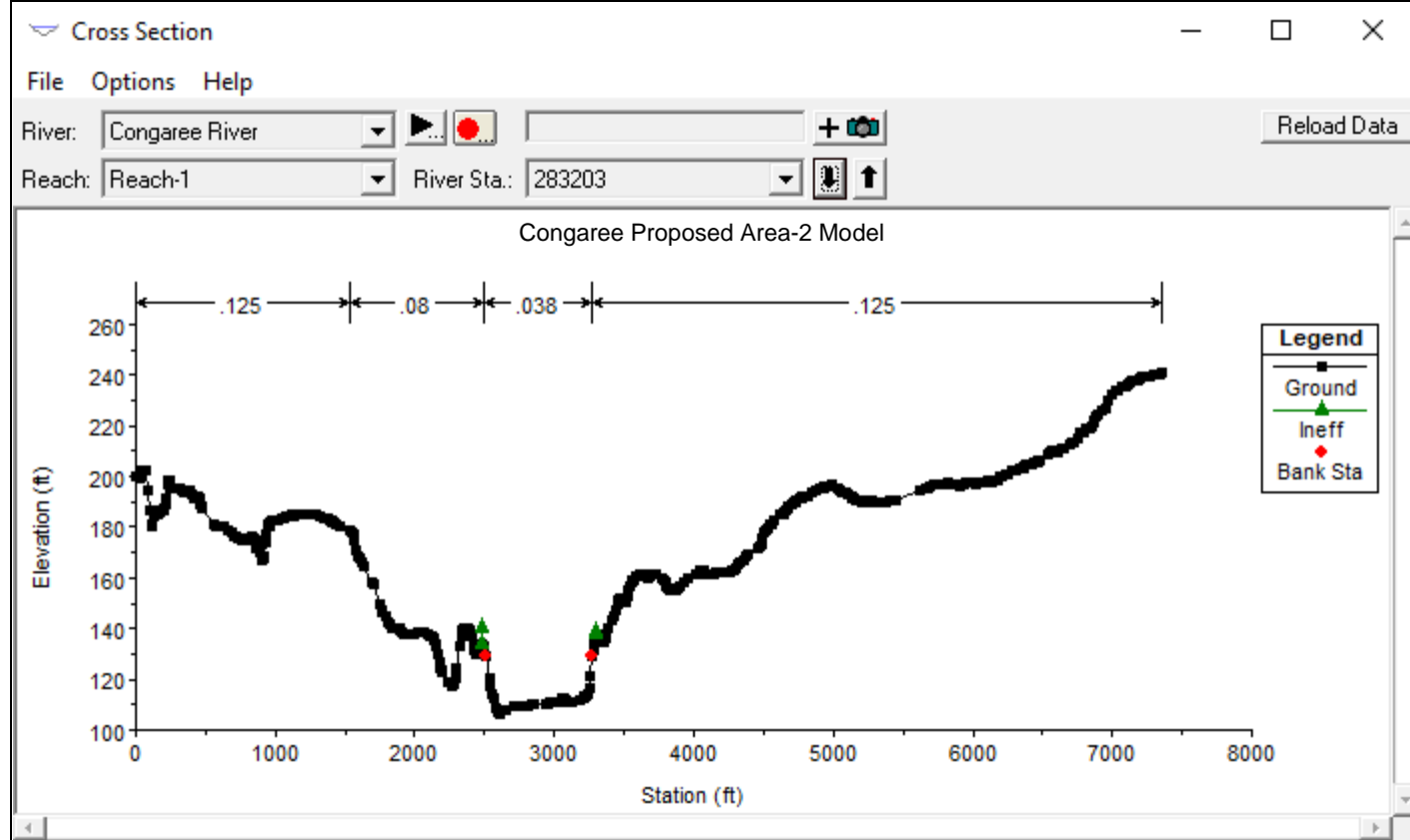
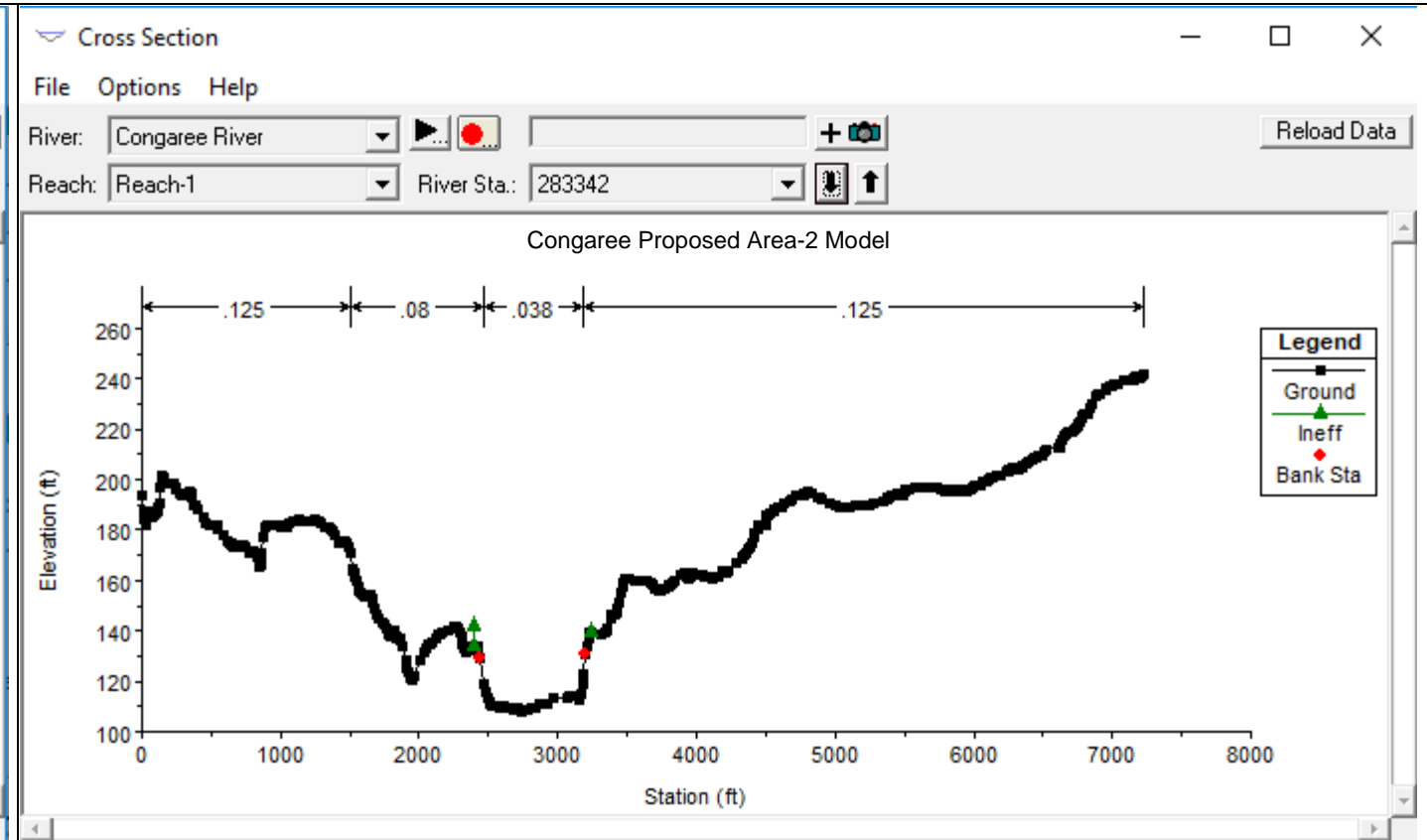
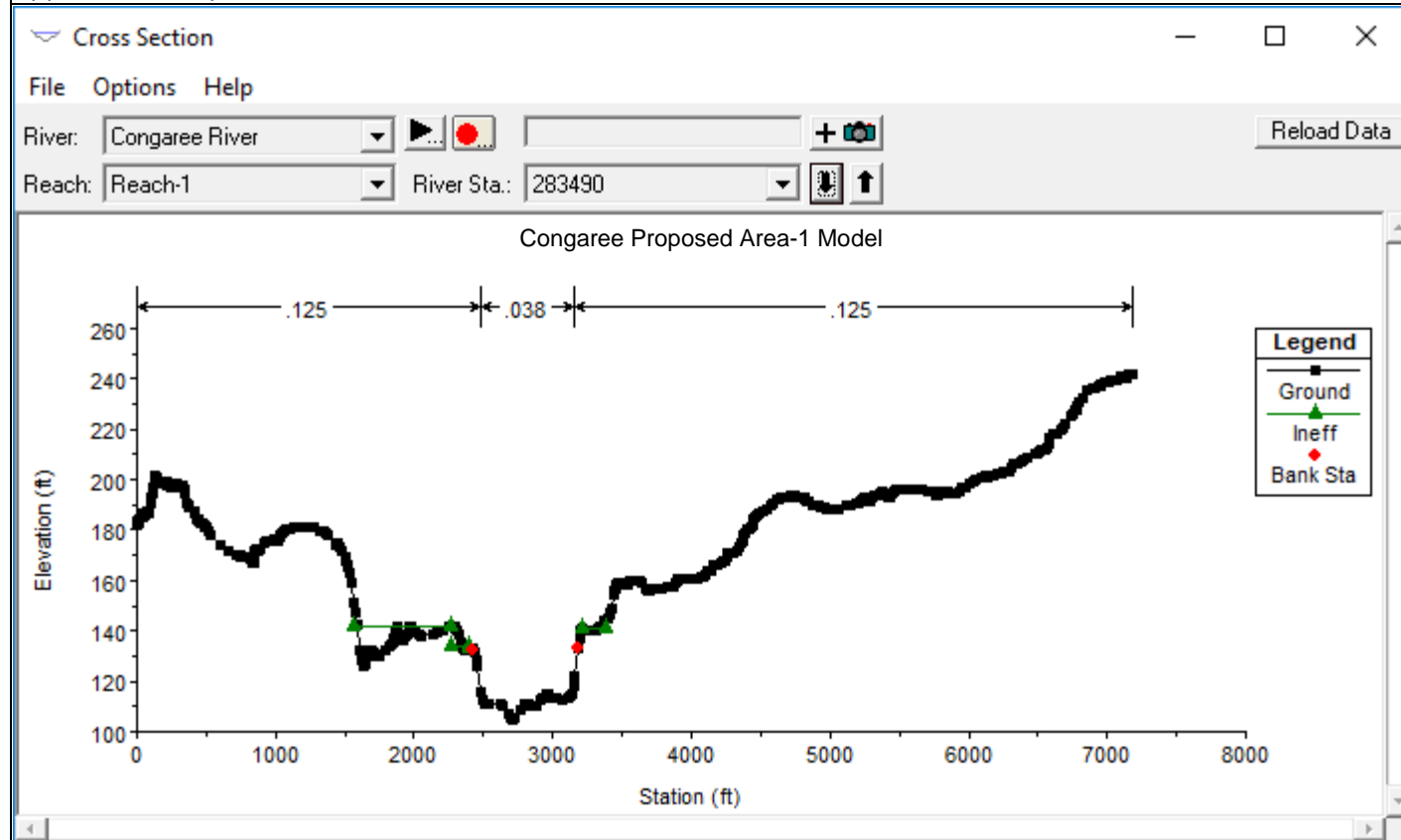




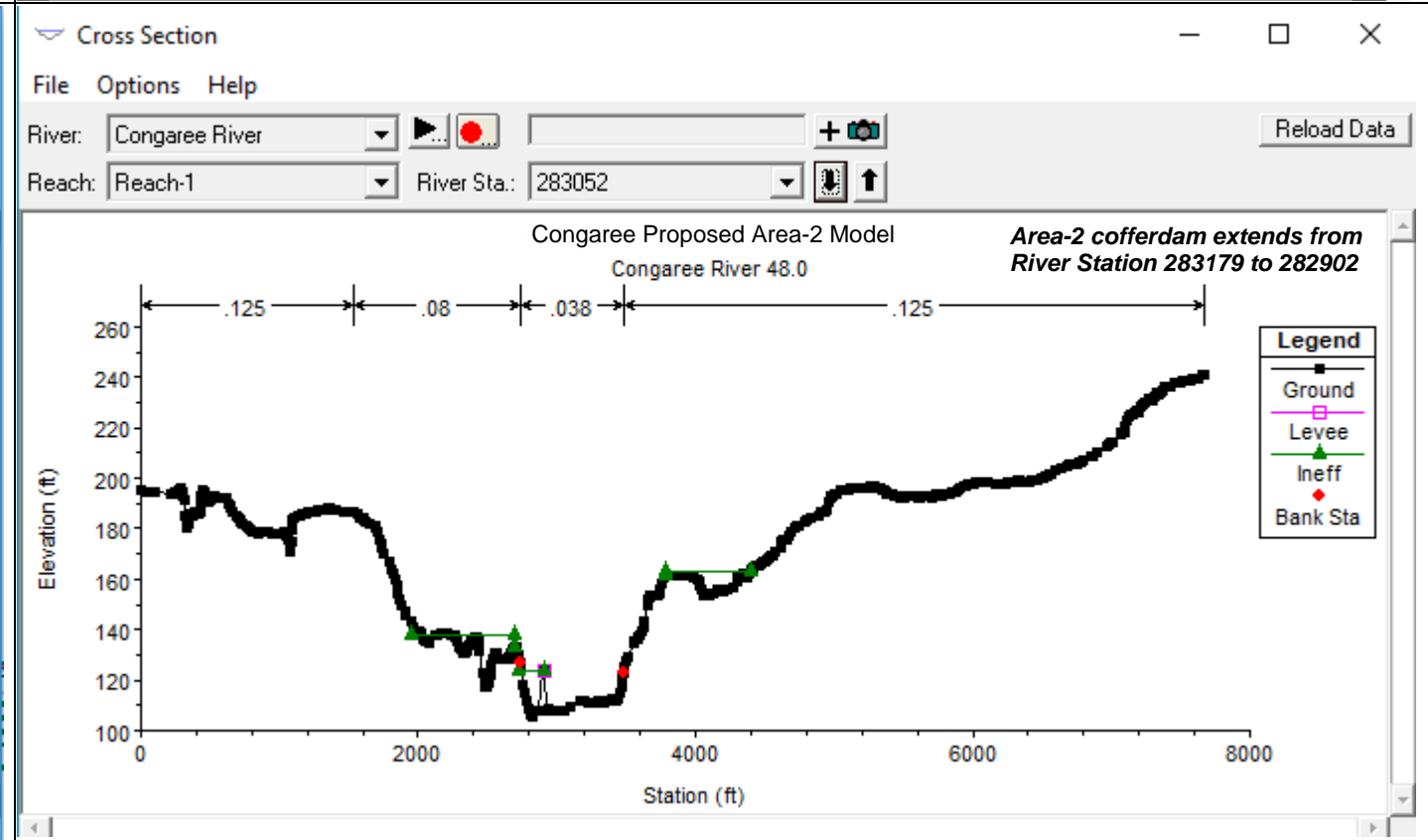
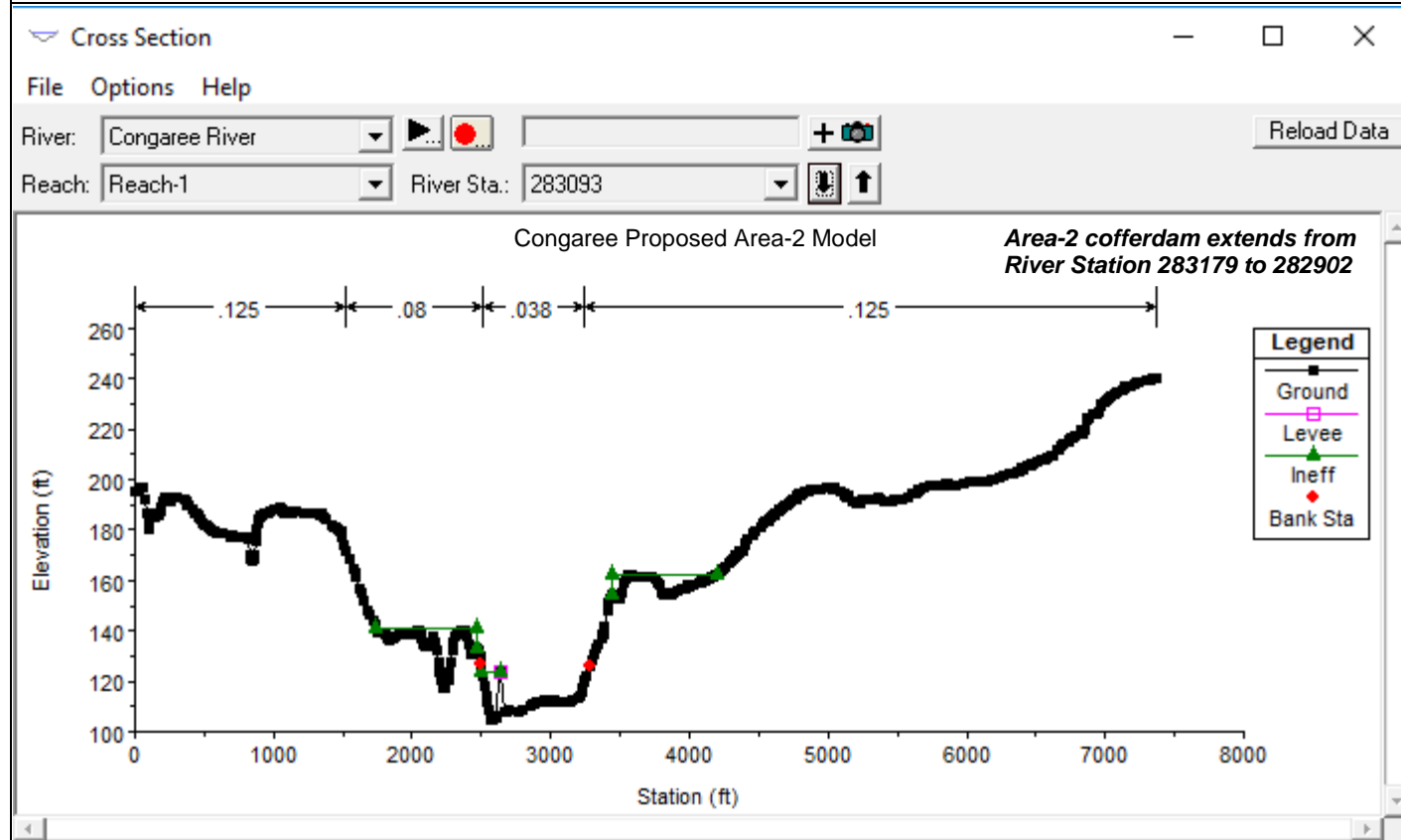
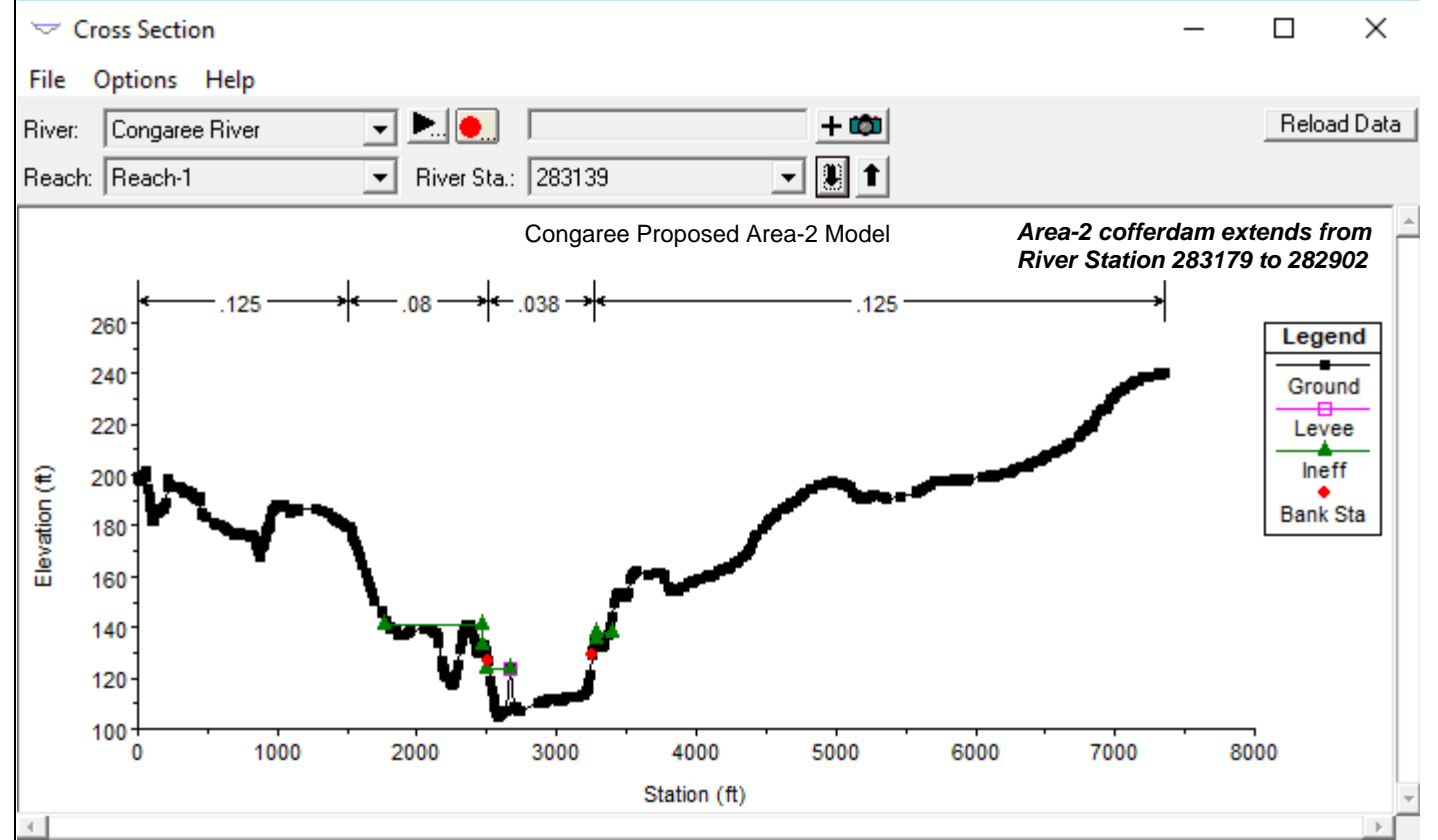
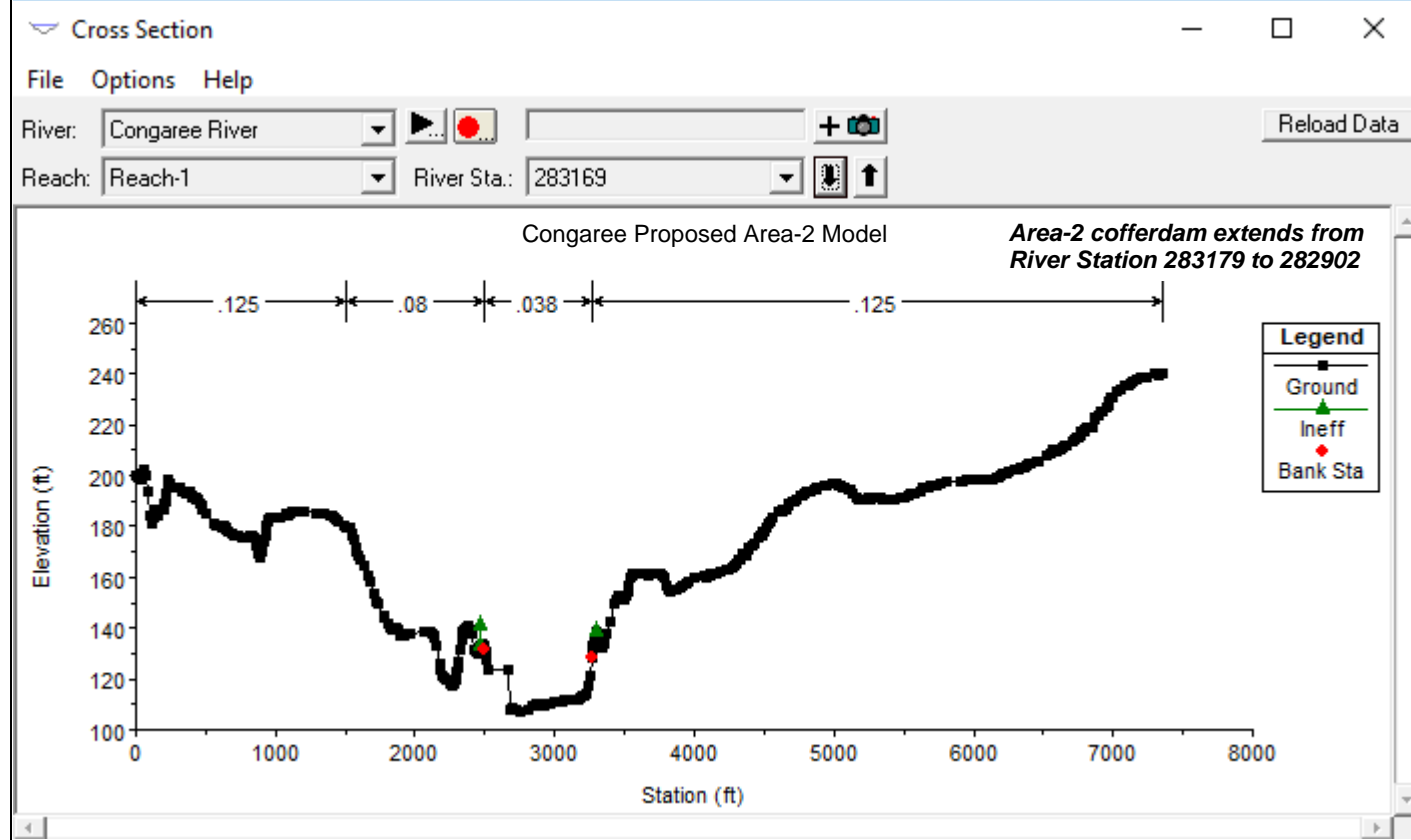
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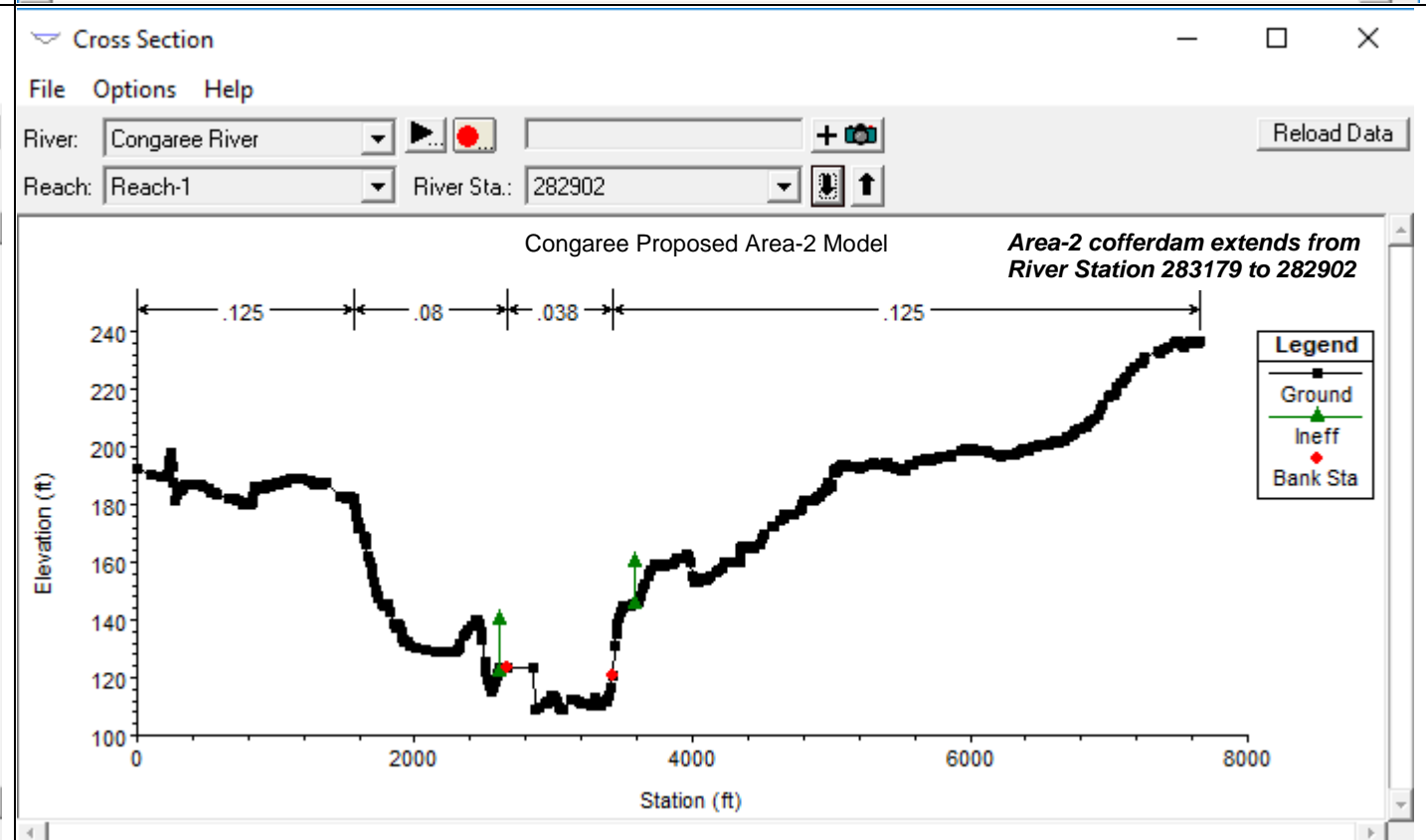
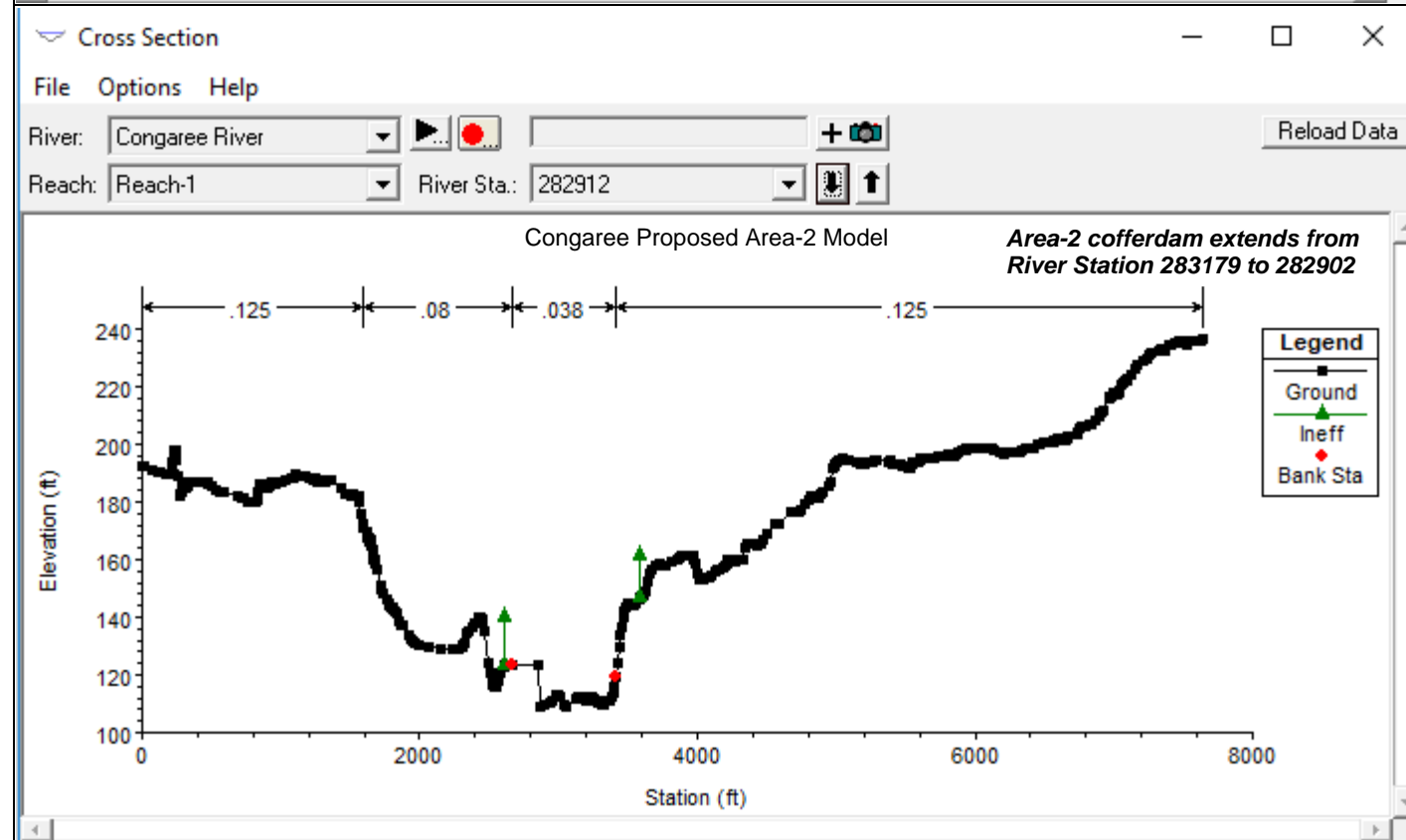
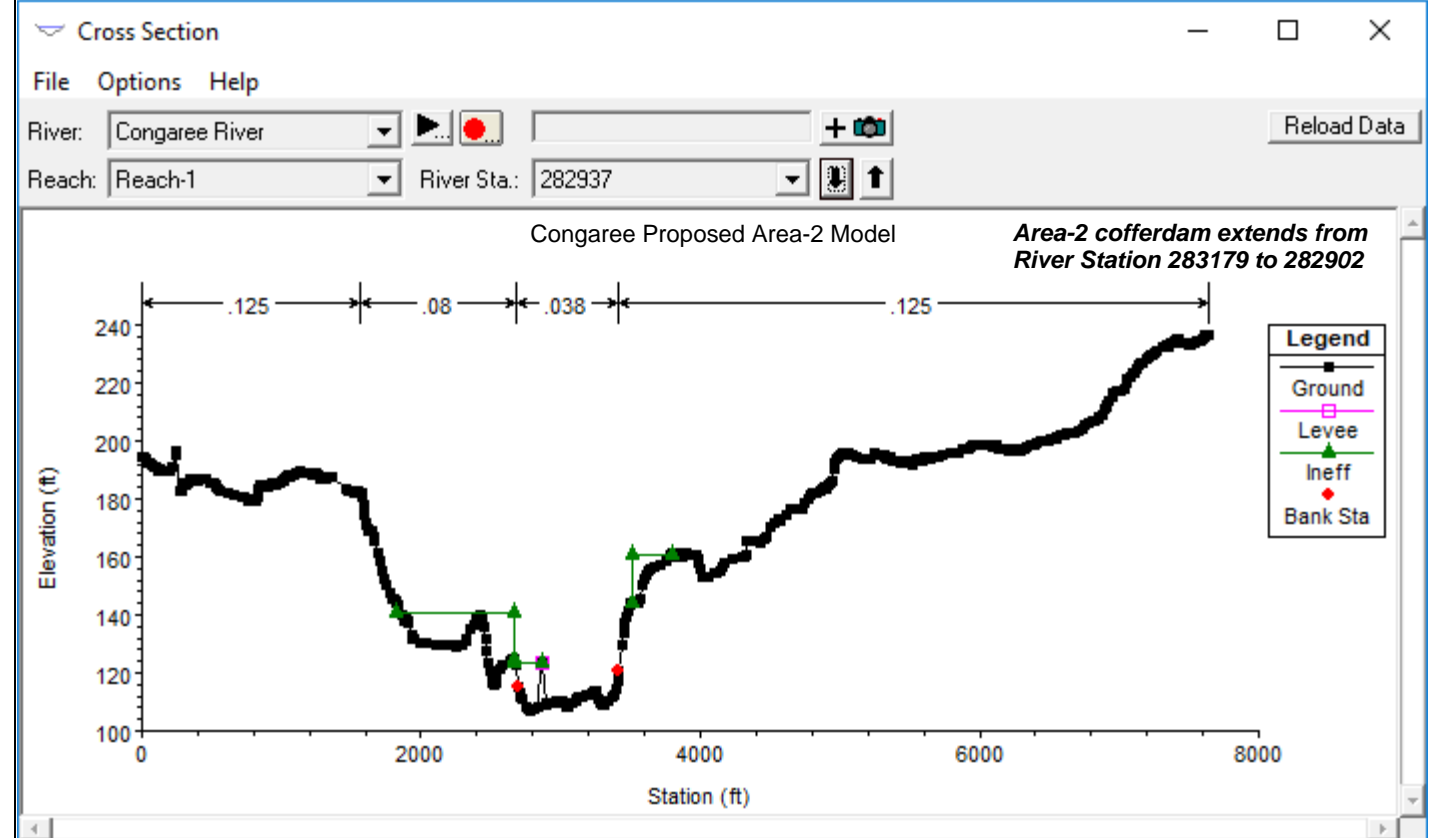
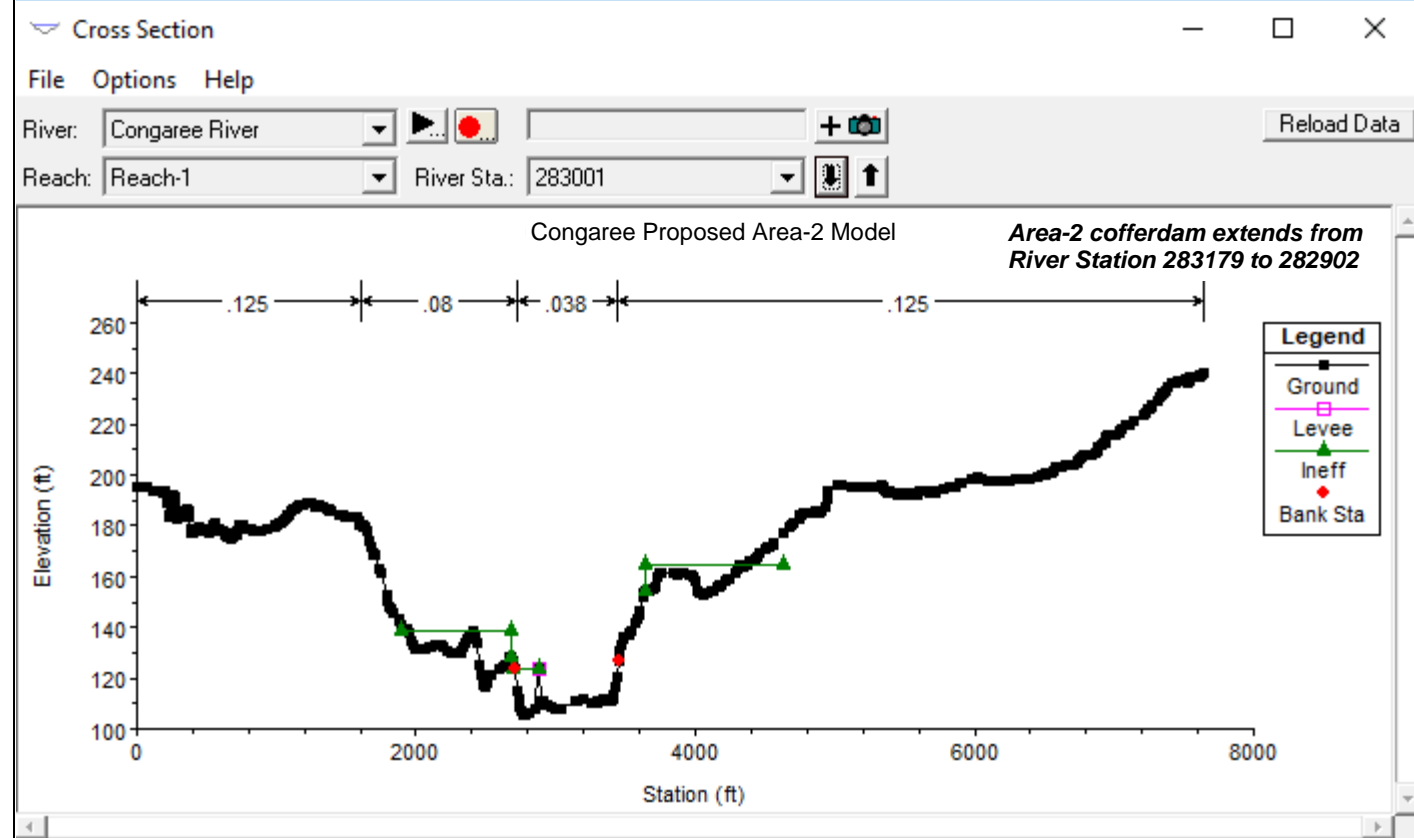
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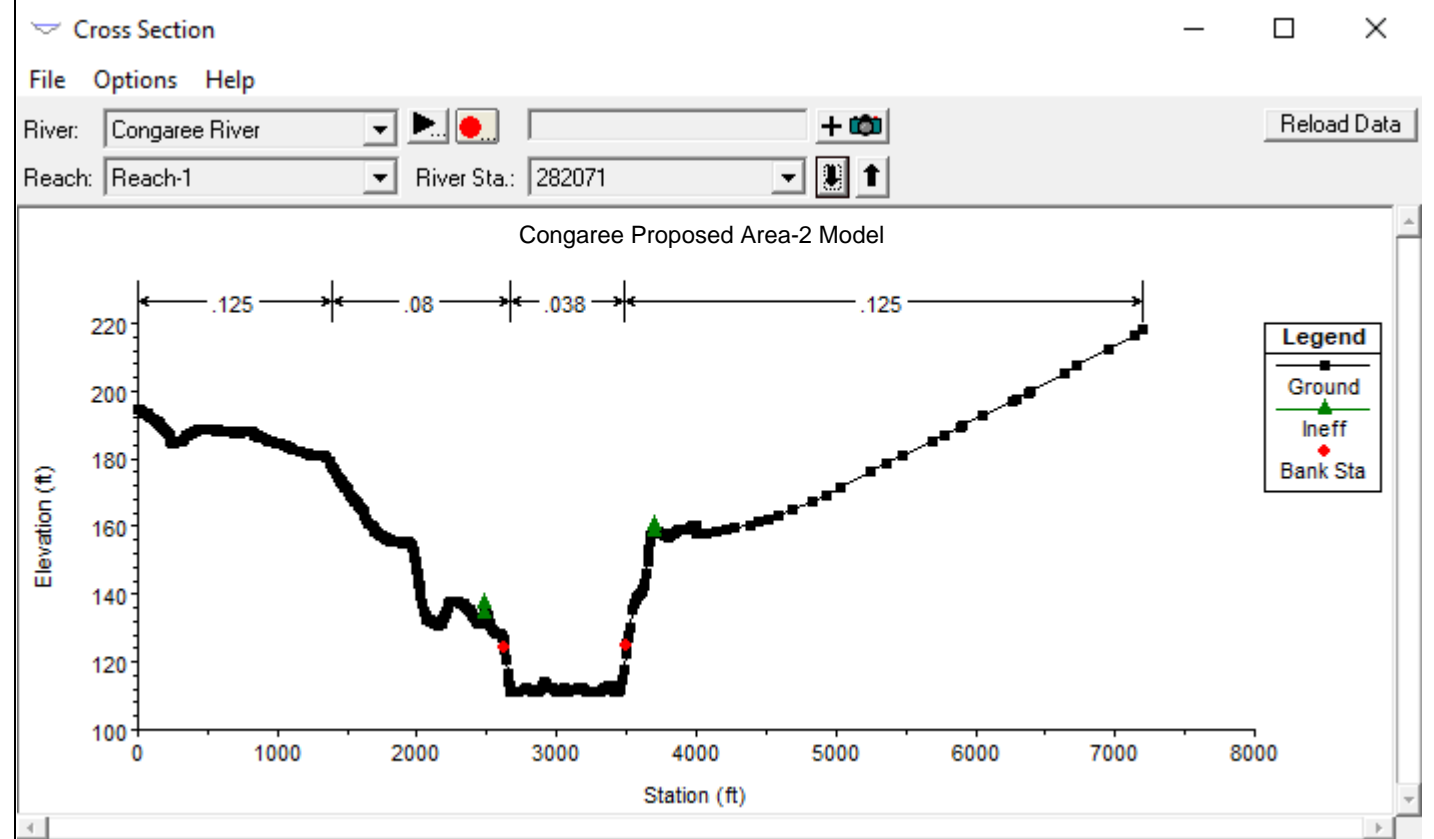
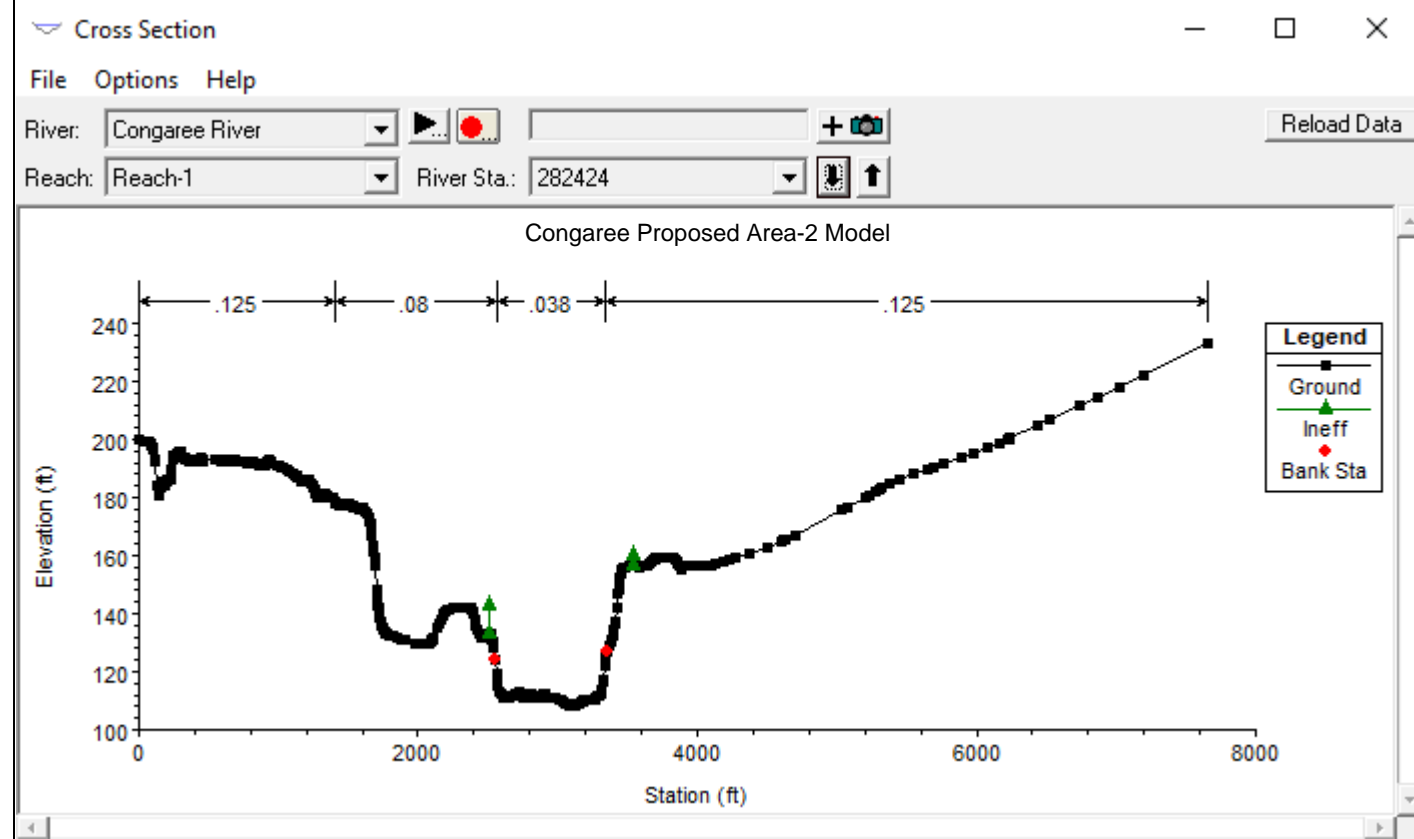
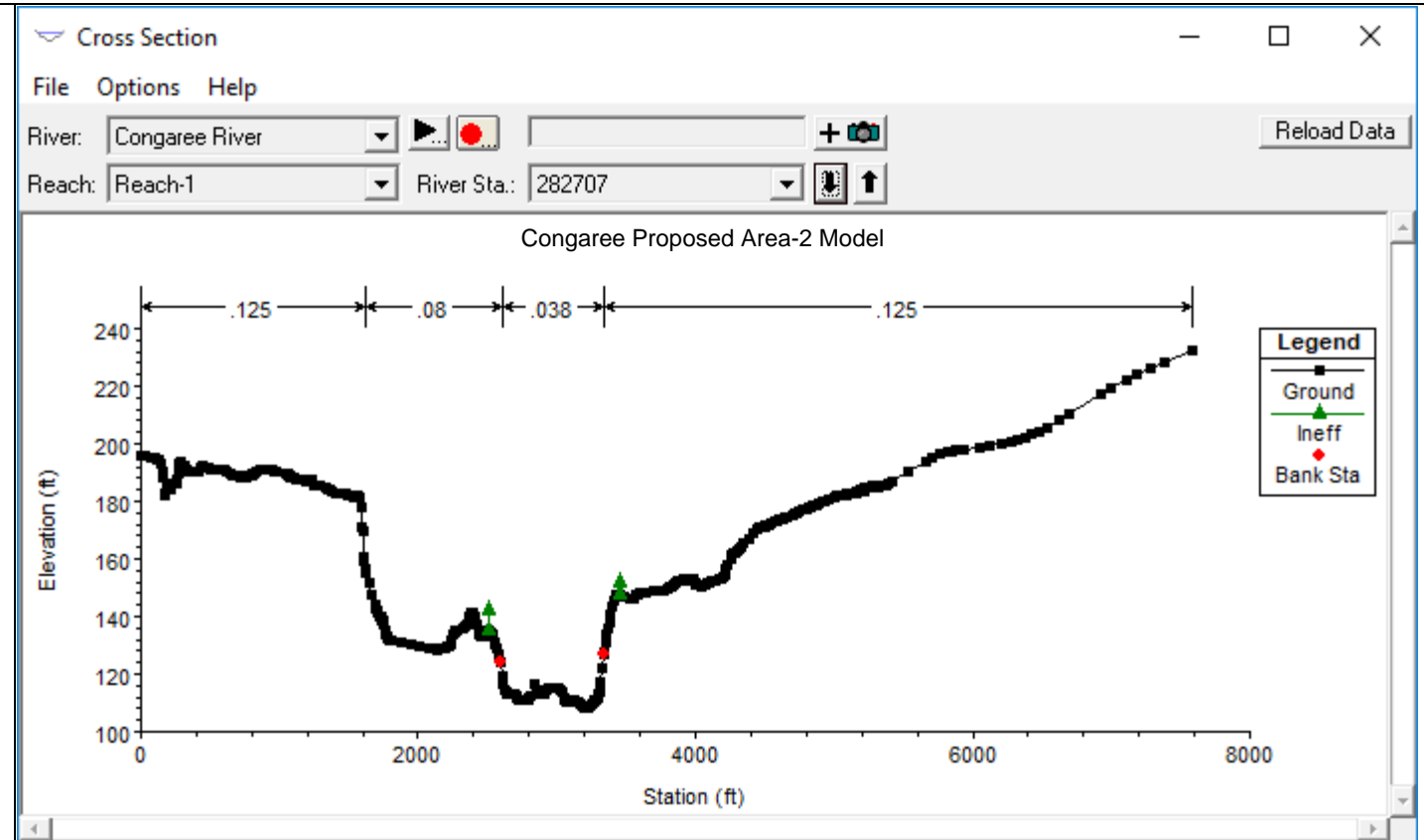
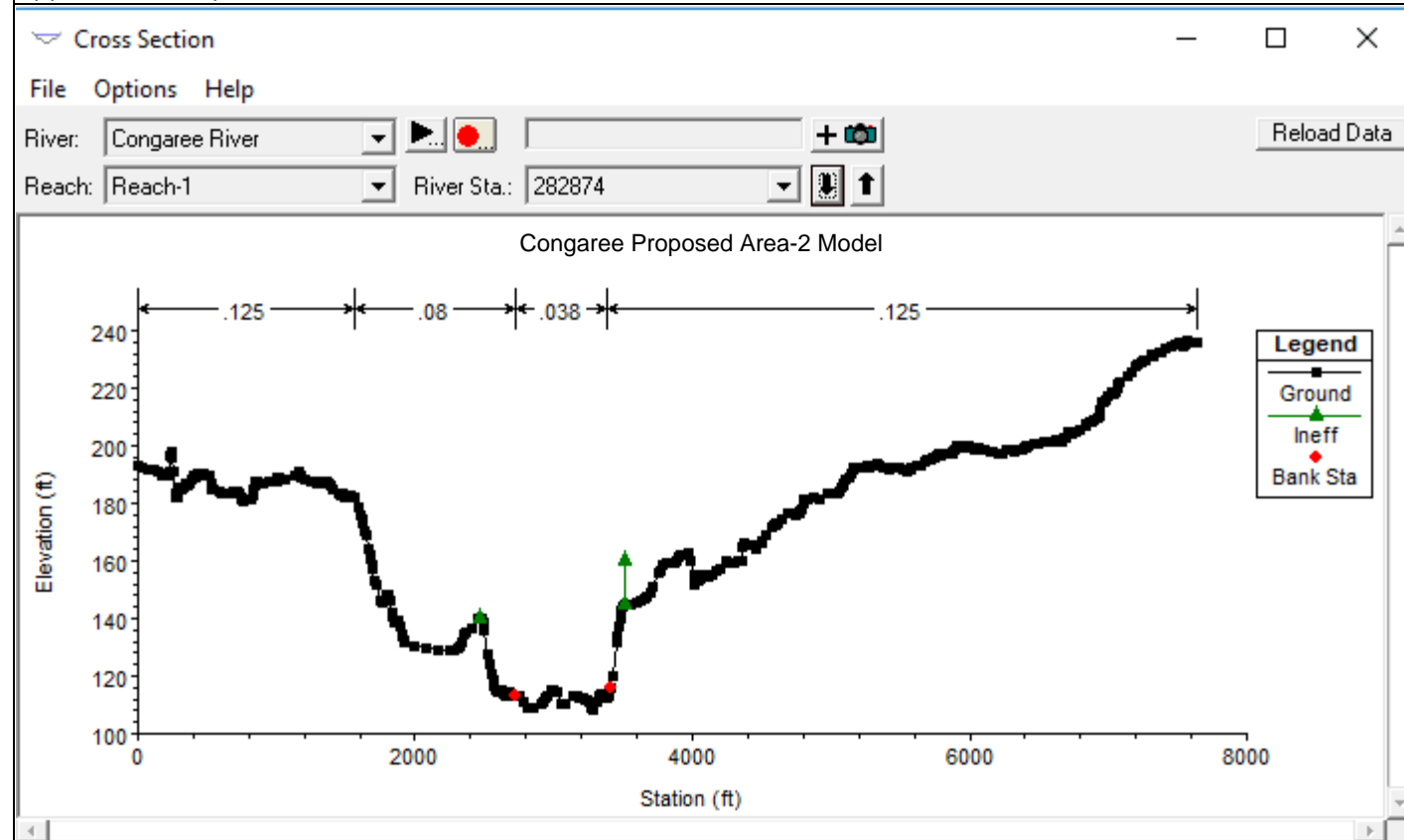
Appendix D: Proposed Conditions Models Cross Sections (Sheet 5 of 8)



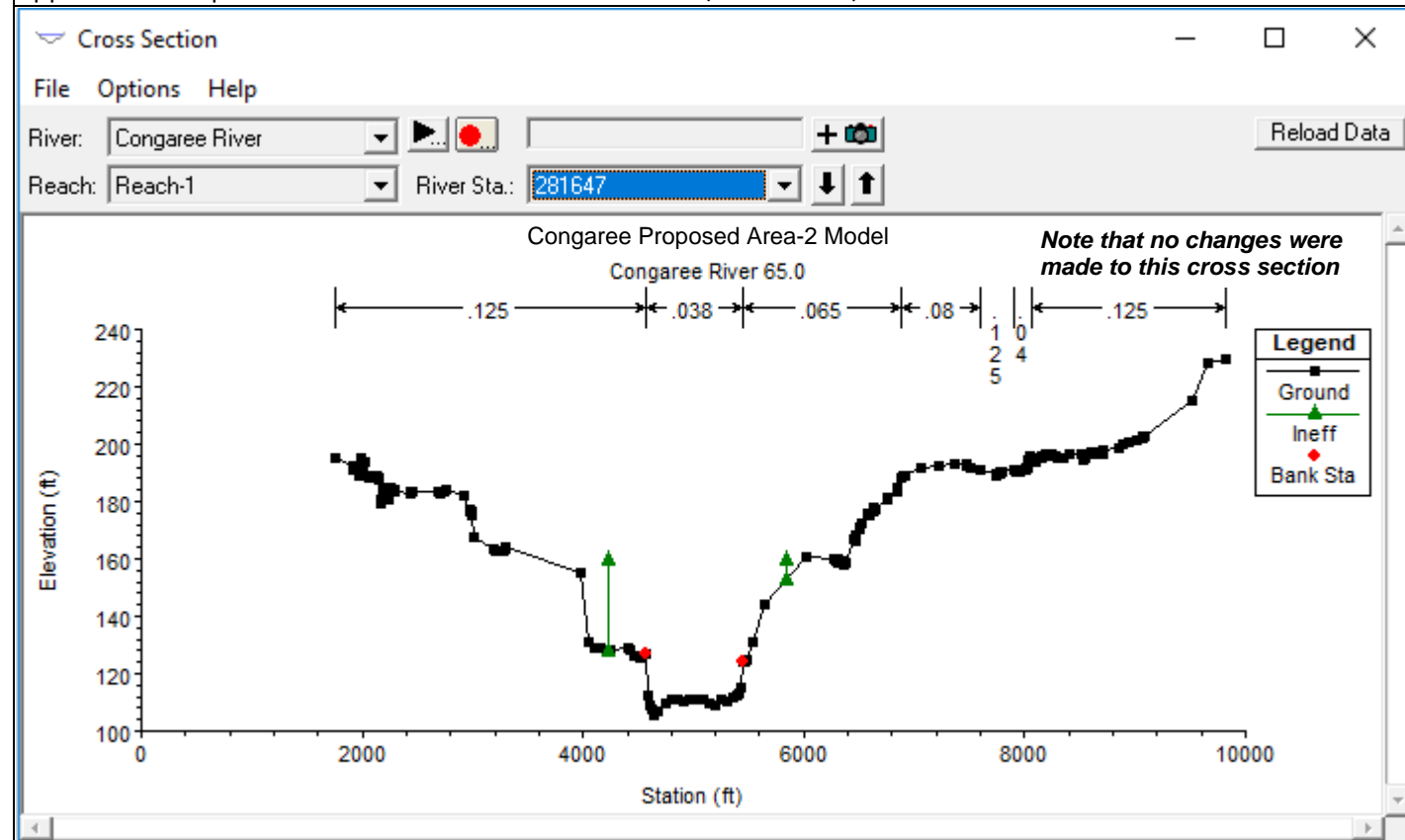
Appendix D: Proposed Conditions Models Cross Sections (Sheet 6 of 8)



Appendix D: Proposed Conditions Models Cross Sections (Sheet 7 of 8)



Appendix D: Proposed Conditions Models Cross Sections (Sheet 8 of 8)





## Appendix E: HEC-RAS Output Tables











HEC-RAS River: Congaree River Reach: Reach-1 Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	217472	Max WS	010yr-CorrectedC	146193.00	91.92	118.856		119.15	0.000447	6.10	99399.62	23068.22	0.25
Reach-1	217472	Max WS	050yr-CorrectedC	237774.00	91.92	121.830		122.08	0.000407	6.42	153134.90	25855.44	0.24
Reach-1	217472	Max WS	100yr-CorrectedC	283154.70	91.92	123.014		123.26	0.000405	6.63	174644.70	26117.52	0.24
Reach-1	216472	Max WS	010yr-CorrectedC	146191.90	91.48	118.365	107.27	118.80	0.000400	6.45	86861.05	25810.94	0.25
Reach-1	216472	Max WS	050yr-CorrectedC	237772.00	91.48	121.364	115.99	121.76	0.000400	7.02	145766.00	27383.30	0.25
Reach-1	216472	Max WS	100yr-CorrectedC	283153.30	91.48	122.551	117.03	122.94	0.000400	7.25	169405.90	27458.83	0.25











HEC-RAS River: Congaree River Reach: Reach-1 Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	217472	Max WS	010yr-ProposedD1	146193.30	91.92	118.856		119.15	0.000447	6.10	99399.62	23068.22	0.25
Reach-1	217472	Max WS	050yr-ProposedD1	237772.00	91.92	121.830		122.08	0.000407	6.42	153136.70	25855.44	0.24
Reach-1	217472	Max WS	100yr-ProposedD1	283156.60	91.92	123.014		123.26	0.000405	6.63	174644.70	26117.52	0.25
Reach-1	216472	Max WS	010yr-ProposedD1	146192.00	91.48	118.365	107.27	118.80	0.000400	6.45	86862.89	25811.07	0.25
Reach-1	216472	Max WS	050yr-ProposedD1	237772.20	91.48	121.364	115.98	121.76	0.000400	7.02	145766.00	27383.30	0.25
Reach-1	216472	Max WS	100yr-ProposedD1	283153.70	91.48	122.551	117.03	122.94	0.000400	7.25	169405.90	27458.83	0.25









HEC-RAS River: Congaree River Reach: Reach-1 Profile: Max WS (Continued)

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	217472	Max WS	010yr-ProposedD2	146193.80	91.92	118.856		119.15	0.000447	6.10	99399.62	23068.22	0.25
Reach-1	217472	Max WS	050yr-ProposedD2	237774.50	91.92	121.830		122.08	0.000407	6.42	153134.90	25855.44	0.24
Reach-1	217472	Max WS	100yr-ProposedD2	283156.70	91.92	123.014		123.26	0.000405	6.63	174644.70	26117.52	0.25
Reach-1	216472	Max WS	010yr-ProposedD2	146192.40	91.48	118.365	107.27	118.80	0.000400	6.45	86862.89	25811.07	0.25
Reach-1	216472	Max WS	050yr-ProposedD2	237772.20	91.48	121.364	115.98	121.76	0.000400	7.02	145766.00	27383.30	0.25
Reach-1	216472	Max WS	100yr-ProposedD2	283153.70	91.48	122.551	117.03	122.94	0.000400	7.25	169405.90	27458.83	0.25

ENGINEERING "NO-RISE" CERTIFICATION

This is to certify that I am a duly qualified engineer licensed to practice in the State of South Carolina.

It is to further certify that the attached technical data supports the fact that proposed Congaree River Remediation Project will  
*(Name of Development)*

not impact the 100-year flood elevations, floodway elevations and floodway widths on Congaree River at published sections  
*(Name of Stream)*

in the Flood Insurance Study for Richland County,  
*(Name of Community)*

dated December 21, 2017 and will not impact the 100-year flood elevations, floodway elevations, and floodway widths at unpublished cross-sections in the vicinity of the proposed development.

Attached are the following documents that support my findings:

- Congaree River Remediation Project Hydraulic Analysis Memo, April 12, 2019
- \_\_\_\_\_
- \_\_\_\_\_
- \_\_\_\_\_
- \_\_\_\_\_

(Date) 4/12/2019

(Signature) John P. Osterle

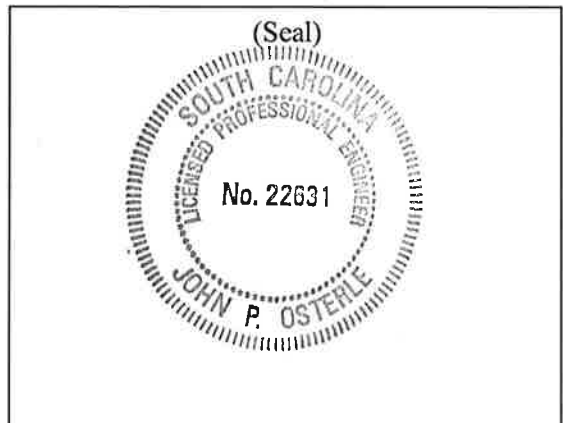
(Title) Project Manager

WSP USA

11 Stanwix, Suite 950

Pittsburgh, PA 15222

(Address)



ENGINEERING "NO-RISE" CERTIFICATION

This is to certify that I am a duly qualified engineer licensed to practice in the State of South Carolina.

It is to further certify that the attached technical data supports the fact that proposed Congaree River Remediation Project will

*(Name of Development)*

not impact the 100-year flood elevations, floodway elevations and floodway widths on Congaree River at published sections

*(Name of Stream)*

in the Flood Insurance Study for Lexington County,

*(Name of Community)*

dated July 5, 2018 and will not impact the 100-year flood elevations, floodway elevations, and floodway widths at unpublished cross-sections in the vicinity of the proposed development.

Attached are the following documents that support my findings:

Congaree River Remediation Project Hydraulic Analysis Memo, April 2019

(Date) 8/13/2019

(Signature) John P. Osterle

(Title) Project Manager

WSP USA

11 Stanwix, Suite 950

Pittsburgh, PA 15222

(Address)

